

PERIODIC SAFETY FACTOR ASSESSMENT 391-3-4-.10(4) AND 40 C.F.R. PART 257.73(e) PLANT MCDONOUGH ASH POND 1 (AP-1) GEORGIA POWER COMPANY

The Federal CCR Rule and the Georgia CCR Rule (391-3-4-.10) require the owner or operator of a CCR surface impoundment to conduct initial and periodic safety factor assessments. See 40 C.F.R. § 257.73(e); Ga. Comp. R. & Regs. r. 391.3-4-.10(4)(b)¹. A direct final rule revision to a partial vacatur of the Final Rule became effective on October 4, 2016. This revision eliminated the exemption for inactive CCR surface impoundments and required such units to meet the same requirements as existing CCR surface impoundments. The owner or operator of the CCR unit must conduct an assessment of the CCR unit and document that the minimum safety factors outlined in 40 C.F.R. § 257.73(e)(1)(i) through (iv) for the embankment are achieved. In addition, the Rules require a subsequent assessment be performed within 5 years of the previous assessment. See 40 C.F.R. § 257.73(f)(3); Ga. Comp. R. & Regs. r. 391.3-4-.10(4)(b) 1.

The CCR surface impoundment known as Ash Pond 1 (AP-1), owned, and operated by Georgia Power Company, is located at Plant McDonough-Atkinson (Plant McDonough) in Cobb County, Georgia. AP-1 no longer receives CCR or other waste streams and no longer functions as a CCR surface impoundment and is in the process of obtaining a solid waste permit under the Georgia Rules for Solid Waste Management, 391-3-4-.10. Installation of the final cover system for Plant McDonough AP-1 was substantially completed Q1 2017, and AP-1 is undergoing additional closure construction in the near term in accordance with 40 C.F.R. § 257.102(d), including the installation of a fully encompassing subsurface barrier wall and adjacent associated closure system upgrades.

AP-1 currently consists of 31 acres of drainage area, and stormwater is routed over the closure system through a system of downslope and perimeter channels to two outfall points: the Northwest and the South outfalls. The current conditions were evaluated for stability under four loading conditions as per 40 CFR §257.73(e):

- Storage Pool (40 C.F.R. § 257.73(e)(i))
- Surcharge Pool (40 C.F.R. § 257.73(e)(ii))
- Seismic Loading Conditions (40 C.F.R. § 257.73(e)(iii))
- Post-Seismic Liquefaction Conditions (when liquefaction susceptible materials are present; 40 C.F.R. § 257.73(e)(iv)).

Engineering analysis of AP-1 in its current condition were evaluated for each loading condition. Stability safety factors were evaluated for each of the loading scenarios using the computer program SLIDE (2018). As required by the EPA rule, a general limit equilibrium (GLE) method (Morgenstern and Price) was used to calculate factors of safety, and the factors of safety were calculated by dividing the resisting forces by the driving forces along the calculated critical slip surface of a given slope.

^[1] In a typographical error, 391.3-4.10(4)(b) references the "structural integrity criteria in 40 CFR 247.73," when the reference to such criteria should be 40 CFR 257.73.

Stability was evaluated along three cross-sections for AP-1 as shown in Figure 1. Subsurface stratigraphy at each cross-section was developed based on a combination of historical site data and subsurface investigations by WSP. Material properties were developed for the dike, foundation, and impounded materials from this data. The conditions modeled in the stability analyses are reflective of the conditions for AP-1 from 2018 through the date of this submittal.

For the surcharge pool scenario, WSP considered the effects of the 100-year 24-hour rain event. This event was calculated to cause temporary water flow on top of the pond cap in drainage channels. Factors of safety for stability under seismic loading conditions were calculated based on the earthquake hazard corresponding to a probability of exceedance of 2% in 50 years (2,475-year return period). WSP used the Bray and Travasarou displacement-based seismic slope stability screening method (Bray and Travasarou 2009) to evaluate the seismic stability. Additionally, an evaluation of the liquefaction susceptibility of the site soils which will remain saturated in the long term was completed and the results incorporated into the post liquefaction stability assessments.

The table below summarizes the results of the slope stability analyses for the current conditions at AP-1, with figures displaying the stability analysis results attached to this demonstration.

2018 to Current Conditions (November 2022 Survey) Stability Analysis Results							
Analysis Case	Storage Pool	Surcharge Pool	Seismic	Post Liquefaction			
Rule Section	§ 257.73(e)(i)	§ 257.73(e)(ii)	§ 257.73(e)(iii)	§ 257.73(e)(iv)			
Target Factor of Safety	1.5	1.4	1.0	1.2			
Cross-Sections	Factor of Safety						
A-A	1.6	1.6	1.5	1.6			
B-B	1.6	1.6	1.3	1.6			
C-C	1.5	1.5	1.4	1.5			

For all cases analyzed, the calculated factors of safety are in excess of those required in Sections § 257.73(e)(i) to (iv) of the EPA Rule.

I certify that the safety factor assessment for AP-1 was conducted in accordance with 40 C.F.R. § 257.73(e).

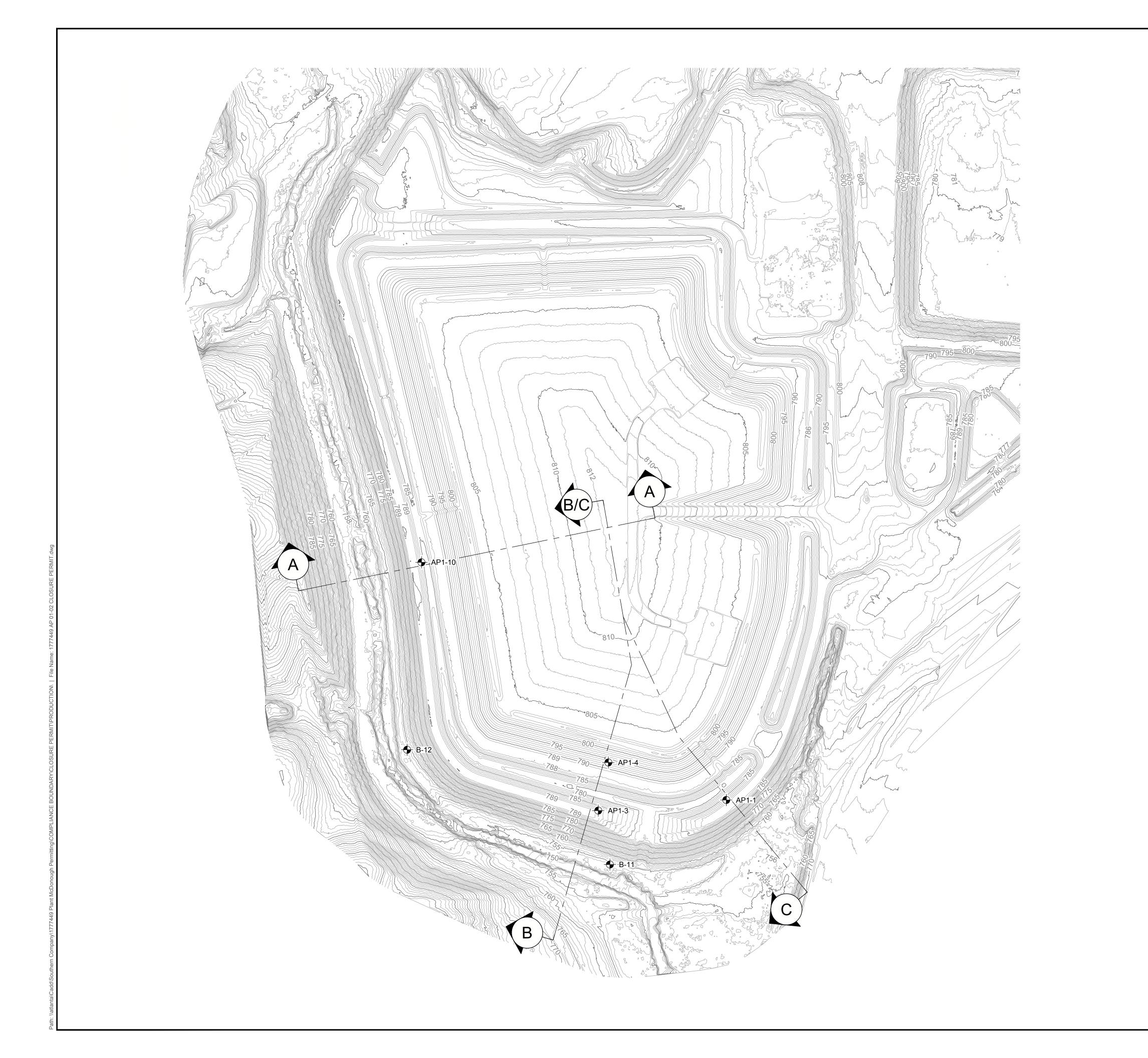


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WSP USA Inc.





LEGEND

EXISTING CONTOURS

SOUTHERN COMPANY BOREHOLES

REFERENCES

1. THE EXISTING TOPOGRAPHY SHOWN EVERYWHERE ELSE WAS PROVIDED BY SOUTHERN COMPANY SERVICES AS AN INTERIM CONSTRUCTION PROGRESS SURVEY. FLOWN ON 04-15-17 USING LIDAR.

2. SOUTHERN COMPANY BOREHOLES COMPLETED IN JANUARY 2009.



GEORGIA POWER COMPANY



PROJECT
PLANT MCDONOUGH SAFETY FACTOR ASSESSMENT

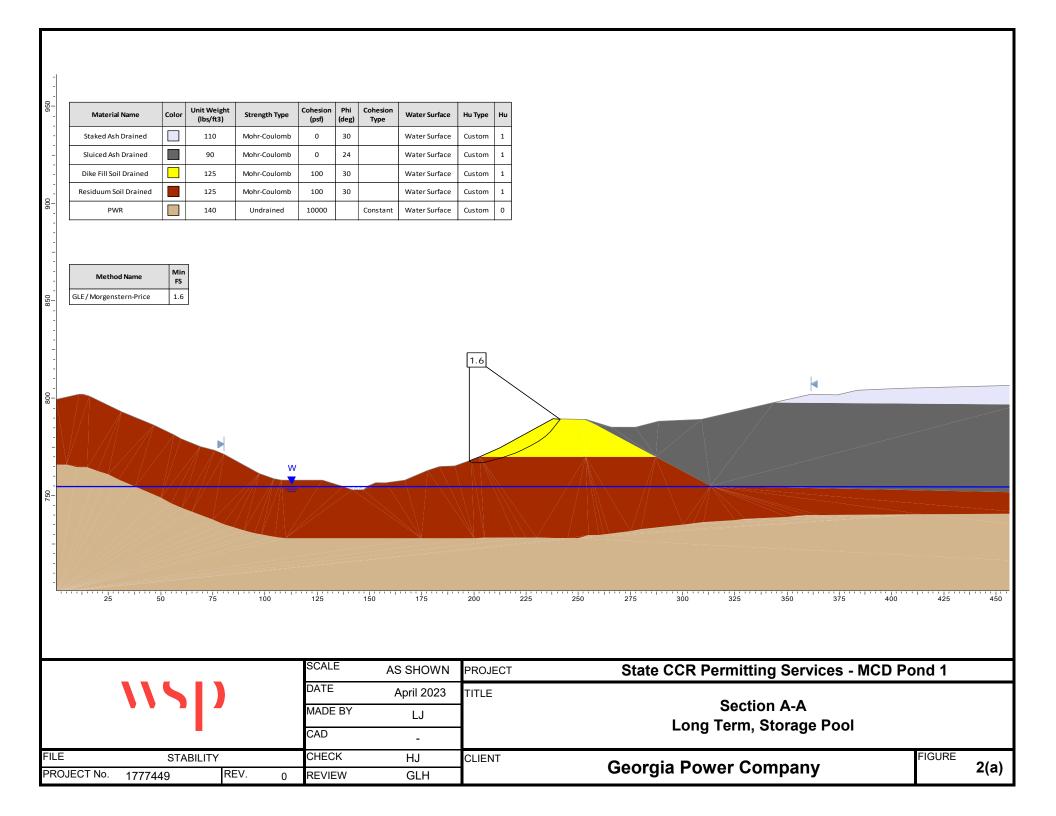
CCR UNIT AP-1 - STABILITY SECTIONS PLAN

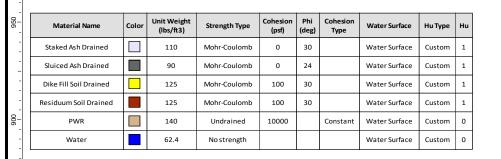
CONSULTANT	

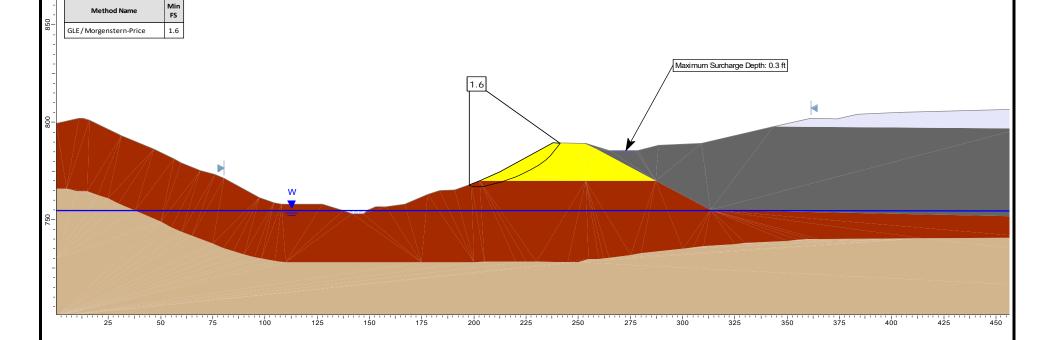
YYYY-MM-DD	2023 APRIL
DESIGNED	LJ
PREPARED	RMS
REVIEWED	JGM/ HJ / LS
ΔPPROVED	GLH

APPROVED REV.

PROJECT NO. 1777449 SHEET







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115])	DATE		April 2023	TITLE	Section A-A				
	M	MADE BY LJ CAD _		1					
•				С	Surchage Pool				
FILE	STABILITY		С	HECK	HJ	CLIENT	Coorgio Dower Company		2/b)
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