INITIAL SAFETY FACTOR ASSESSMENT
40 C.F.R § 257.100(f)(2)(iv) and 40 C.F.R. § 257.73(e)
PLANT MITCHELL ASH POND 1 (AP-1)
GEORGIA POWER COMPANY

A rule amendment to the Federal Coal Combustion Residuals (CCR) Rule (40 C.F.R. Part 257) became effective on November 8, 2024. *See* Hazardous and Solid Waste Management System: Disposal of Coal Combustion Residuals from Electric Utilities; Legacy CCR Surface Impoundments, 89 Fed. Reg. 38950 ("Legacy Rule"). The Legacy Rule defines the term "legacy CCR surface impoundment" and establishes regulatory requirements for units that meet the definition of a legacy CCR surface impoundment. The Legacy Rule requires the owner or operator of a legacy CCR impoundment to conduct initial and periodic safety factor assessments for each CCR unit and document whether the calculated factors of safety for each unit achieve the minimum safety factors specified in 40 C.F.R. § 257.73(e)(1)(i) through (iv) for the critical section of the dam. In addition, the Rules require a subsequent assessment to be performed within 5 years of the previous assessment [*See* 40 C.F.R. § 257.73(f)(3)].

The legacy CCR surface impoundment known as Plant Mitchell Ash Pond 1 (AP-1) is located approximately 8 miles south of Albany, Georgia on Georgia Power's Plant Mitchell property. AP-1 is approximately 53 acres and was formed by constructing an engineered perimeter dam around the impoundment. The Notification of Intent to Initiate Closure of AP-1 was placed in the Operating Record in October 2019. AP-1 no longer receives CCR and is currently undergoing closure by removal wherein CCR material is removed and transported by rail and truck for beneficial use or disposal at an approved landfill.

Slope stability analyses were performed for four critical sections through the perimeter dike slope of AP-1 as included in the attached calculation package. The analyses of the four critical sections (A-A', B-B', C-C' and D-D') resulted in the following minimum safety factors:

Loading Condition	Long-term Maximum Storage Pool (Static)	Maximum Surcharge Pool (Static)	Seismic
Rule Section	§ 257.73(e)(1)(i)	§ 257.73(e)(1)(ii)	§257.73(e)(1)(iii)
Minimum Required Safety Factor	1.5	1.4	1.0
Cross Sections	***Minimum Calculated Safety Factor***		
A-A'	2.7	2.7	2.1
B-B'	1.6	1.6	1.2
C-C'	1.6	1.6	1.1
D-D'	1.6	1.5	1.1

The soils used to construct the AP-1 perimeter dike are not susceptible to liquefaction, primarily due to the low magnitude of seismic shaking anticipated at the site and due to the predominantly clayey soils with significant fines fraction used to construct the embankment. After review of the limited available data, simplified screening for cyclic mobility indicates that the dike is not susceptible to cyclic mobility during seismic events. Therefore, evaluation of minimum liquefaction factor of safety per §257.73(e)(1)(iv) is not required.

This assessment is supported by appropriate engineering calculation package which is attached.

I hereby certify that the safety factor assessment was conducted in accordance with 40 C.F.R. § 257.100(f)(2)(iv) and 40 C.F.R. § 257.73.

Ramil Garcia Mijares, Ph.D., P.E.
Licensed State of GA, PE No. PE042944

Attachment: Calculation Package

PLANT MITCHELL ASH POND 1 (AP-1) CALCULATION PACKAGE

PREPARED BY: PG
REVIEWED BY: AF
APPROVED BY: RM

1 INTRODUCTION

This calculation package presents the results of the slope stability analysis performed to evaluate the initial factor of safety for Plant Mitchell Ash Pond 1 (AP-1) dikes in accordance with of the Federal CCR Rule (40 C.F.R. § 257) which requires the owner or operator of a legacy CCR surface impoundment to conduct initial and periodic safety factor assessments.

Several subsurface investigations have been conducted at AP-1 and Plant Mitchell Ash Pond 2 (AP-2) at various times in the past. The investigations consisted of auger borings with standard penetration tests (SPT), cone penetration tests (CPT) borings, and test pits. Laboratory tests were conducted on select samples which informed geotechnical material properties for different materials at the site. This calculation package also aims to refine these properties for use in the stability analysis based on available SPT data.

2 METHODOLOGY

WSP analyzed four critical cross-sections (A-A' through D-D') in AP-1 as shown in Appendix A. These locations were selected based on the steepness and height of the dike, presence of any known historical instabilities, and proximity to water storage structures like storage lagoon. Subsurface stratigraphy for each cross-section was developed based on the historical subsurface data and the aerial survey conducted by WSP in January 2025 (See Appendix B). Material properties were developed for the dike, residual/existing, and CCR materials based on these data.

WSP used a two-dimensional general limit equilibrium approach for the analysis. The Morgenstern-Price method was used which satisfies both force and moment equilibrium and which considers both shear and normal interslice forces. Analysis was performed using Rocscience's SLIDE2 (2023) computer program. Non-circular failure surfaces with a minimum depth of 5 ft deep were modeled for each cross-section, and critical failure surfaces corresponding to the lowest factor of safety are presented in this calculation package. AP-1 was analyzed using topographic conditions based on January 2025 survey data and factors of safety were evaluated under three loading conditions: (i) long-term maximum storage pool (static); (ii) maximum surcharge pool (static); and (iii) seismic.

The long-term maximum storage pool was established using the maximum historical groundwater elevation recorded between 2016 and 2025. Refer to Section 4 for details on the groundwater elevations for the selected critical cross-sections. Only cross-section A-A' contains free surface water (Refer to Appendix D) to an elevation of 187 ft corresponding to the design water level of the lined Contact Water Lagoon. Currently, with ongoing closure activities at AP-1, WSP anticipates that the groundwater levels presented in Section 4 represent maximum storage pool conditions.

AP-1 is designed to store runoff from a 1,000-year, 24-hour storm event with a storm depth of 12 inches, therefore, a maximum surcharge pool of 12 inch was considered in the analysis. For seismic evaluation, the Hynes-Griffin & Franklin (1984) pseudo-static screening method was used with a pseudo-static coefficient equal to one half of the Peak Ground Acceleration (PGA) at the site. A PGA value of 0.065g was adopted based on the

earthquake hazard corresponding to 2% in 50 years (2,475-year return period) probability of exceedance using United States Geological Society Hazard Tool[1]. The shear strength parameters of the soil were reduced by 20% to account for dynamic excess pore pressures during seismic loading. The materials below groundwater table were modeled as undrained, while those above water were modeled as drained under seismic loading, with strength reductions applied per Haynes-Griffin & Franklin (1984). Furthermore, liquefaction susceptibility was assessed using Bray and Sancio (2006) method based on available Atterberg Limits data.

The following geotechnical investigation data, reports, design calculations, and records were reviewed for this analysis. Relevant excerpts were included in Appendix B.

- Southern Company Services Inc., Earth Science and Environmental Engineering, Log of Test Boring, March 2010, (SCS, 2010).
- Wood Environment & Infrastructure Solutions, Inc., Hydrogeologic Assessment Report and Conceptual Site Model, Plant Mitchell – Ash Ponds, A, 1 & 2, Dougherty and Mitchell Counties, November 2018, (Wood, 2018).
- Wood Environment & Infrastructure Solutions, Inc., Geotechnical Site Characterization Report, February 8, 2019, (Wood, 2019a).
- Wood Environment & Infrastructure Solutions, Inc., Calculations – Slope Stability, Beneficial Use Collection – Truck on Dike, March 13, 2019, (Wood, 2019b).
- Wood Environment & Infrastructure Solutions, Inc., Calculations – Slope Stability, Beneficial Use Collection – Truck on Haul Road on Ash, March 15, 2019, (Wood, 2019c).
- Wood Environment & Infrastructure Solutions, Inc., Annual and Semi-Annual Groundwater Monitoring and Corrective Action Report, Plant Mitchell – Ash Ponds, A, 1 & 2, 2019-2022, (Wood, 2019-2022).
- WSP USA Environment & Infrastructure, Inc., (2016–2025). GW elevations Aug 2016 to Sept 2025 [Microsoft Excel spreadsheet], Unpublished internal document. (WSP, 2016-2025).
- WSP USA Environment & Infrastructure, Inc., Annual and Semi-Annual Groundwater Monitoring and Corrective Action Report, Plant Mitchell – Ash Ponds, A, 1 & 2, 2023-2025, (WSP, 2023-2025).
- Charah Solutions/Geocomp, Monthly Instrumentation Report, Georgia Power- Plant Mitchell, Ash Pond Closure Project, July, August, September 2024 and April, June, July 2025, (Charah/Geocomp, 2024-2025).

3 MATERIAL PROPERTIES

The SPT blow counts and material classification for AP-1 and AP-2 dike materials from SCS (2010) are summarized in Appendix C. Both dikes consist of similar soil types, ranging from sandy fat clays and lean clays to clayey sands and silty sands. However, AP-1 dike materials generally have lower SPT blow counts compared to AP-2 dike materials, indicating comparatively weaker strength properties. Triaxial tests results are available only for AP-2 dike and residual materials from SCS (2010). Hence, it was necessary to evaluate and adopt appropriate shear strength parameters for AP-1 dike stability analyses. Figure 3-1 below shows a comparison between different shear strength envelopes – SCS (2010) triaxial tests results and Wood (2019a) recommended shear strength parameters – available to use for AP-1 stability analyses.

[1] https://earthquake.usgs.gov/nshmp/

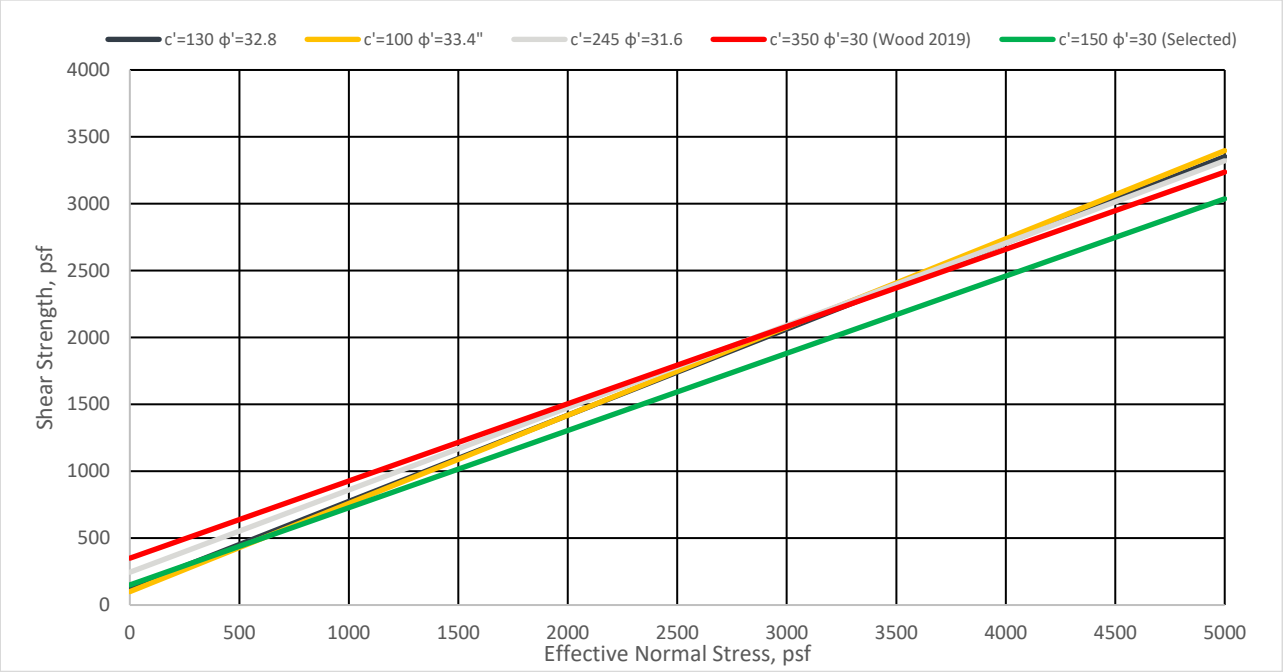


Figure 3-1: Comparison between different shear strength envelopes

A lower conservative Mohr-Coulomb shear strength envelope, shown as green line in Figure 3-1, was selected for the AP-1 dike materials. For the CCR material, despite being suggested in Wood (2019a) no distinction in unit weights was made between drained and undrained loading conditions. The underlying bedrock consists of Ocala Limestone which was assigned infinite strength for modeling purposes due to its higher strength relative to overlying soils. Similarly, a lower conservative Mohr–Coulomb shear strength envelope was adopted for residual soils based on triaxial test results from SCS (2010). The material properties used in the stability analyses are summarized below.

Table 3-1: Material Properties

Material Type	Unit Weight (pcf)	Static			Pseudo-Static[6]		
		ϕ' (degrees)	c (psf)	s_u (psf)	ϕ' (degrees)	c (psf)	s_u (psf)
Dike Material[1]	125	30	150	1,200	24.8	120	960
CCR Material	70[2]	24	0	200	19.6	0	160
Residual/Existing Soil[3]	120	18	250	1,000[4]	14.6	200	800
Bedrock[5]	125	-					

1- Lower conservative shear strength envelope as shown in Figure 3-1.
2- Unit weights during drained and seismic loading considered same.
3- Minimum shear strength envelope selected from SCS (2010) investigation.
4- Minimum value adopted from the CPT data analysis from Wood (2019a).
5- Infinite strength is assigned only for purpose of analysis.
6- Per Hynes-Griffin & Franklin (1984) method with 20% reduction in peak strength.

4 WATER LEVEL ELEVATIONS

Groundwater elevations were generally selected from piezometers located in or near the vicinity of each analysis section. For each section, groundwater elevations were selected at three locations: upstream, near the toe, and downstream of the dike. Maximum groundwater levels were obtained from one of the following reports or monitoring data: Wood (2019-2020), WSP (2016-2025), WSP (2023-2025), and Charah/Geocomp (2024-2025). Where piezometer data were unavailable, groundwater elevations were estimated based on the general trend of groundwater level, or maximum potentiometric surface elevation, or the elevation at the closest analysis section.

WSP assumed that groundwater levels would be maintained at or below these selected elevations at the AP-1 dikes; however, if groundwater levels rise above these elevations, re-evaluation of the stability analyses is recommended. The groundwater elevations used in the analysis are presented in Table 4-1.

Table 4-1: Ground Water Elevations Selected for Analysis

Stability Analysis Section	Upstream of the Dike (ft)	Near Toe of the Dike (ft)	Downstream of the Dike (ft)	Nearby Piezometer
A-A'	183.0	176.0[1]	167.8	Upstream: MIT_CH _AP1_PZ-04 Downstream: PZ-4S
B-B'	185.0	148.0[2]	145.7	Upstream: MIT_CH _AP1_PZ-15 Downstream: PZ-02R
C-C'	179.0	147.4	146.4	Upstream: MIT_CH_AP1-0-DPZ-12 Near Toe: PZ-18 Downstream: PZ-10S
D-D'	179.0[3]	147.5[1]	147.5	Downstream: MW-116

1- Based on the general trend of groundwater level.
2- Based on the maximum potentiometric surface elevation.
3- Based on groundwater level in nearby section C-C'.

5 RESULTS

The SLIDE outputs showing the failure surface with lowest factor of safety for each loading condition and section are provided in Appendix D. A summary of the minimum factor of safety from the stability analyses is shown in table below:

Table 5-1: Calculated Slope Stability Factor of Safety

Loading Condition	Long-term Maximum Storage Pool (Static)	Maximum Surcharge Pool (Static)	Seismic
Rule Section	§ 257.73(e)(1)(i)	§ 257.73(e)(1)(ii)	§257.73(e)(1)(iii)
Minimum Required Safety Factor	1.5	1.4	1.0
Cross Sections	***Minimum Calculated Safety Factor***		
A-A’	2.7	2.7	2.1
B-B’	1.6	1.6	1.2
C-C’	1.6	1.6	1.1
D-D’	1.6	1.5	1.1

The AP-1 dike is not susceptible to flow liquefaction primarily due to the low magnitude of seismic shaking anticipated (PGA=0.065g). In addition, the dike materials – ranging from sandy fat clays and lean clays to clayey sands and silty sands – contain a significant fines fraction (approximately 46%) which reduces susceptibility to flow liquefaction. Furthermore, a simplified screening for susceptibility to cyclic mobility, based on Bray and Sancio (2006), was conducted. Available data were limited; only one data point for sandy lean clay (water content= 16.7%, liquid limit=43, and plasticity index=24) from SCS (2010) was plotted. This plots within the “Not Susceptible” zone (See Appendix E), indicating non-susceptibility to cyclic mobility during seismic shaking. Therefore, evaluation of factor of safety for liquefaction per §257.73(e)(1)(iv) is not required.

6 CONCLUSION

The stability analyses show that the analyzed cross-sections exceed the minimum target factor of safety required per § 257.73(e)(1). Furthermore, AP-1 dike materials are also not susceptible to liquefaction.

7 REFERENCES

Bray, J. D., & Sancio, R. B. (2006). Assessment of the liquefaction susceptibility of fine-grained soils. Journal of Geotechnical and Geoenvironmental Engineering, 132(9), 1165–1177.

Charah Solutions/Geocomp. (2024–2025). Monthly instrumentation report, Georgia Power – Plant Mitchell, Ash Pond Closure Project (July, August, September 2024 and April, June, July 2025).

Hynes-Griffin, M. E., & Franklin, A. G. (1984). Rationalizing the seismic coefficient method (Misc. Paper GL-84-13). U.S. Army Engineer Waterways Experiment Station.

Southern Company Services Inc., Earth Science and Environmental Engineering. (2010). Log of Test Boring.

Wood Environment & Infrastructure Solutions, Inc. (2018). Hydrogeologic assessment report and conceptual site model, Plant Mitchell – Ash Ponds A, 1 & 2, Dougherty and Mitchell Counties.

Wood Environment & Infrastructure Solutions, Inc. (2019a). Geotechnical site characterization report.

Wood Environment & Infrastructure Solutions, Inc. (2019b). Calculations – Slope stability, beneficial use collection – Truck on dike.

Wood Environment & Infrastructure Solutions, Inc. (2019c). Calculations – Slope stability, beneficial use collection – Truck on haul road on ash.

Wood Environment & Infrastructure Solutions, Inc. (2019–2022). Annual and semi-annual groundwater monitoring and corrective action report, Plant Mitchell – Ash Ponds A, 1 & 2.

WSP USA Environment & Infrastructure, Inc. (2016–2025). GW elevations Aug 2016 to Sept 2025 [Microsoft Excel spreadsheet]. Unpublished internal document.

WSP USA Environment & Infrastructure, Inc. (2023–2025). Annual and semi-annual groundwater monitoring and corrective action report, Plant Mitchell – Ash Ponds A, 1 & 2.

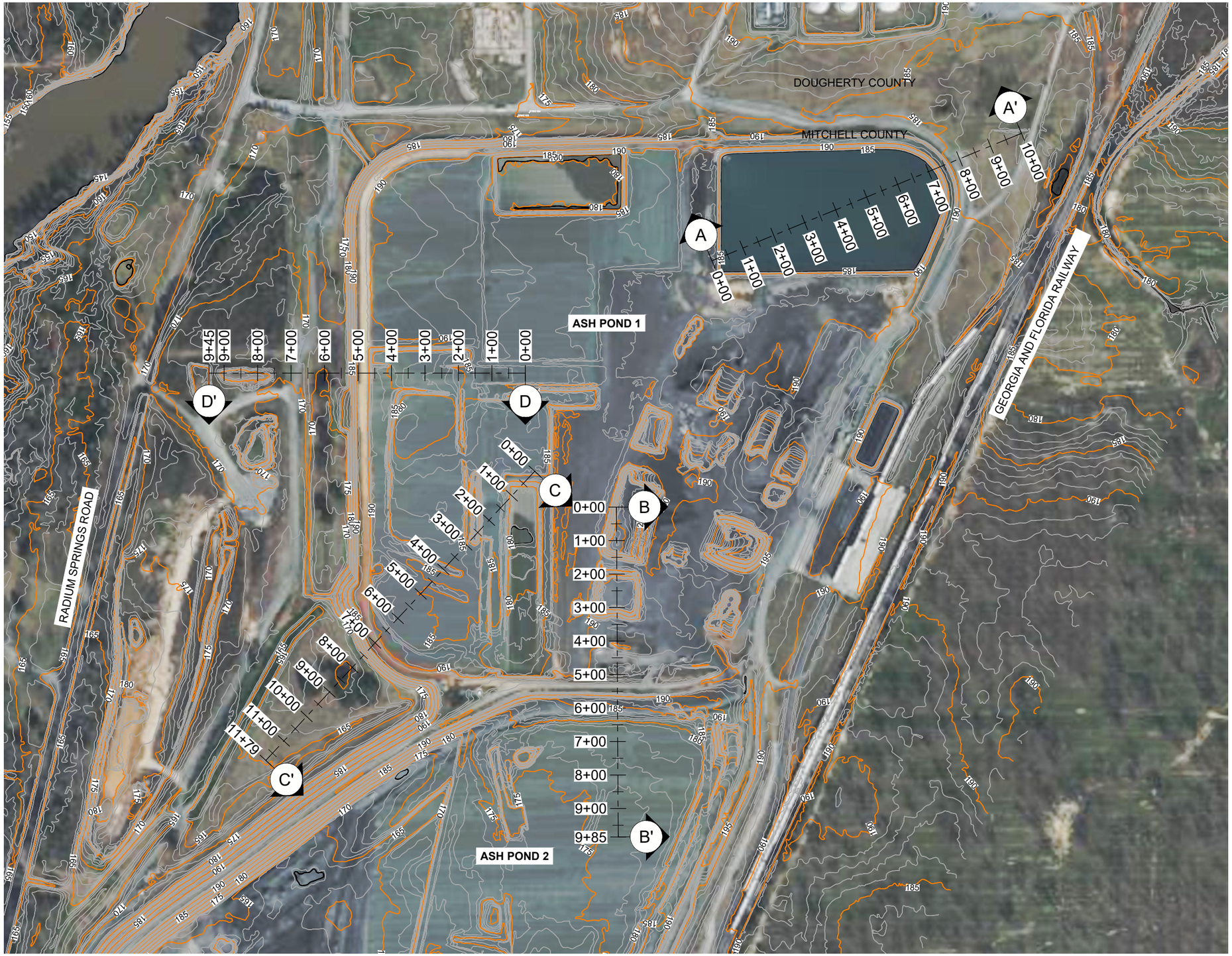
8 ATTACHMENTS

The following are attached to this calculation package:

Appendix A – Location Plan and Section
Appendix B – Background Information
Appendix C – SPT Blow Counts Comparison
Appendix D – Slope Stability Model Outputs
Appendix E – Liquefaction Screening

APPENDIX A

Location Plan and Section



KEYMAP

REFERENCE

1. BASE MAP TAKEN FROM ©2025 Microsoft Corp ©2025 Maxar© CNES(2025) Distribution Airbus DS Microsoft Bing, DATED 10/2025.
2. Contours from the 2025 January Aerial Survey by WSP.

0 150 300
1" = 300' FEET

PLANT MITCHELL
SITE PLAN WITH SECTION LOCATIONS - AERIAL

Prepared By: CP
Reviewed By: PG
Approved By: RM

Date: 10/20/2025
WSP Project No: US0037149.0170

Path: C:\Users\USCP701386\Desktop\TEMP\Plant Mitchell_Prakath\ | File Name: US0037149.0170 Plant Mitchell .dwg

APPENDIX B

Background Information

SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/24/2010 **COMPLETED** 3/24/2010 **SURF. ELEV.** 185.8 **COORDINATES:** N 524,385.52 E 2,306,317.50
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** G. Wilson **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 36.5 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Ash - North Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		(Fly ash)		SS -1	2.5-4.0	1-1-1 (2)		
5				SS -2	4.5-6.0	2-2-2 (4)		
				SS -3	7.5-9.0	2-1-2 (3)		
10				UD -4	9.5-11.5			
15				SS -5	14.5-16.0	1-WH-1 (1)		
		MIT-API 037						
20		CONFIDENTIAL BUSINESS INFORMATION		SS -6	19.5-21.0	1-1-2 (3)		
			161.3					
25		Very soft, brown, clayey SAND (SC)		SS -7	24.5-26.0	WH-WH-2 (2)		

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Very soft, brown, clayey SAND (SC) *(Con't)*						
			156.3					
30		Medium dense, brown/white, clayey SAND/weathered limestone		SS -8	29.5-31.0	3-4-7 (11)		
35				UD -9	34.5-36.5			
			149.3					
		Bottom of borehole at 36.5 feet.						
40								
45								
50								
55								

CONFIDENTIAL BUSINESS INFORMATION

SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/30/2010 **COMPLETED** 3/31/2010 **SURF. ELEV.** 191.1 **COORDINATES:** N 524,558.81 E 2,306,368.70
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** J. Jordan **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 46 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Crest - North Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Red, medium dense, CLAYEY SAND (SC)						
				SS -1	2.5-4.0	5-6-7 (13)		
			186.6					
5		Red to black, stiff, SANDY LEAN CLAY (CL)		SS -2	4.5-6.0	4-5-3 (8)		
		Firm, reddish orange		SS -3	7.5-9.0	4-4-3 (7)		
10				UD -4	9.5-11.5			
			176.6					
15		Red to dark gray, medium dense, fine CLAYEY SAND (SC)		SS -5	14.5-16.0	5-6-8 (14)		
20		Loose		SS -6	19.5-21.0	5-4-4 (8)		
25		With black ash		SS -7	24.5-26.0	6-6-5 (11)		CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Red to dark gray, medium dense, fine CLAYEY SAND (SC) *(Con't)*						
			161.6					
30		Stiff, light gray, SANDY FAT CLAY (CH)		SS -8	29.5-31.0	3-4-6 (10)		
35		Yellowish brown, firm, less sand		SS -9	34.5-36.0	2-2-3 (5)		
40				SS -10	39.5-41.0	3-4-6 (10)		
45			145.6	SS -11	44.5-46.0	3-3-4 (7)		
		White, weathered LIMESTONE	145.1					
		Bottom of borehole at 46.0 feet.						
50								
55								

CONFIDENTIAL BUSINESS INFORMATION

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BORING AP1-3
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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/30/2010 **COMPLETED** 3/30/2010 **SURF. ELEV.** 170.7 **COORDINATES:** N 524,600.63 E 2,306,365.71
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** J. Jordan **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 46 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Toe - North Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Loose, dark brown to gray, fine CLAYEY SAND (SC)						
				SS -1	2.5-4.0	3-4-5 (9)		
5		Gray to tan, mottled		SS -2	4.5-6.0	3-3-4 (7)		
		Orange and gray, medium dense		SS -3	7.5-9.0	4-6-9 (15)		
10				UD -4	9.5-11.5			
15		Light gray		SS -5	14.5-16.0	3-3-7 (10)		
20		Loose, light gray to orange		SS -6	19.5-21.0	3-3-6 (9)		
			146.2					
25		White, yellow and gray		SS -7	24.5-26.0	3-4-4 (8)		CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		White, yellow and gray *(Con't)*						
30		Purple, gray and brown, soft SANDY FAT CLAY (CH)		SS -8	29.5-31.0	WH-WH-4 (4)		
			136.2					
35		White, sandy, weathered LIMESTONE		SS -9	34.5-36.0	4-5-3 (8)		
40				SS -10	39.5-41.0	6-12-15 (27)		
45			124.7	SS -11	44.5-46.0	WH-5-7 (12)		
		Bottom of borehole at 46.0 feet.						
50								
55								

CONFIDENTIAL BUSINESS INFORMATION

GEOTECH ENGINEERING LOGS - ESEE DATABASE.GDT - 5/7/10 09:20 - T:\ESEE MAJOR PROJECTS\PROJECTS\MITCHELL STEAM PLANT\2010\ASH POND DIKE\LOGS.GPJ

BORING AP1-4
PAGE 1 OF 2

SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/30/2010 **COMPLETED** 3/30/2010 **SURF. ELEV.** 166.6 **COORDINATES:** N 523,067.93 E 2,306,173.33
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** J. Jordan **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 51 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Toe - West Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Grayish brown, loose, fine CLAYEY SAND (SC)						
				SS -1	2.5-4.0			
			162.1					
5		Light brown, soft, SANDY LEAN CLAY (CL)		SS -2	4.5-6.0			
			159.1					
		Light gray to dark gray, we, very loose, fine SAND (SP), with clay		SS -3	7.5-9.0			
10				UD -4	9.5-11.5			
15		With coarse sand and pebbles		SS -5	14.5-16.0			
			151.1					
		Tan to orange, loose, fine to coarse SAND (SW)						
20				SS -6	19.5-21.0			
			142.1					
25		Firm, red to tan, SANDY LEAN CLAY (CL)		SS -7	24.5-26.0			

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Firm, red to tan, SANDY LEAN CLAY (CL) *(Con't)*						
30		Soft		SS -8	29.5-31.0			
			132.1					
35		Red to tan, loose CLAYEY SAND (SC)		SS -9	34.5-36.0			
40				SS -10	39.5-41.0			
45				SS -11	44.5-46.0			
50		Medium dense	116.1	SS -12	49.5-51.0			
		White, sandy weathered LIMESTONE	115.6					
		Bottom of borehole at 51.0 feet.						
55								

CONFIDENTIAL BUSINESS INFORMATION

GEOTECH ENGINEERING LOGS - ESEE DATABASE.GDT - 5/7/10 09:20 - T:\ESEE MAJOR PROJECTS\PROJECTS\MITCHELL STEAM PLANT\2010\ASH POND DIKE\LOGS.GPJ

BORING AP1-5
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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/23/2010 **COMPLETED** 3/23/2010 **SURF. ELEV.** 190.7 **COORDINATES:** N 523,089.01 E 2,306,231.67
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** G. Wilson **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 81 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Crest - West Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Medium stiff, red, sandy lean CLAY (CL)		SS -1	2.5-4.0	3-3-5 (8)		
5		Soft		SS -2	4.5-6.0	2-2-3 (5)		
			183.2					
		Medium stiff, brown, silty SAND (SM)		SS -3	7.5-9.0	3-2-4 (6)		
10				UD -4	9.5-11.5			
			176.2					
15		Medium stiff, red, lean CLAY (CL)		SS -5	14.5-16.0	5-3-4 (7)		
20				SS -6	19.5-21.0	3-3-3 (6)		
25		Stiff		SS -7	24.5-26.0	5-5-6 (11)		CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Medium stiff, red, lean CLAY (CL) *(Con't)*						
			161.2					
30		Stiff, mottled red/brown/gray, fat CLAY (CH)		SS -8	29.5-31.0	3-4-6 (10)		
			156.2					
35		Very stiff, gray, sandy CLAY (CL)		SS -9	34.5-36.0	6-8-11 (19)		
			151.2					
40		Very loose, red, clayey SAND (SC)		SS -10	39.5-41.0	WH-WH-WH (0)		
45				SS -11	44.5-46.0	WH-WH-3 (3)		
			141.2					
50		Medium stiff, red, sandy CLAY (CL)		SS -12	49.5-51.0	2-3-4 (7)		
			136.2					
55		Loose, red, clayey SAND (SC)		SS -13	54.5-56.0	2-3-5 (8)		

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
60		Loose, red, clayey SAND (SC) *(Con't)* Medium dense		SS -14	59.5-61.0	3-5-9 (14)		
			126.2					
65		Medium stiff, brown, sandy CLAY (CL)		SS -15	64.5-66.0	1-3-5 (8)		
70		Very soft		SS -16	69.5-71.0	WH-WH-WH (0)		
			116.2					
75		White, very loose, clayey sand/weathered limestone		SS -17	74.5-76.0	WH-WH-WH (0)		
80			109.7	SS -18	79.5-81.0	9-11-22 (33)		
		Bottom of borehole at 81.0 feet.						
85								
90								

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/24/2010 **COMPLETED** 3/24/2010 **SURF. ELEV.** 187.4 **COORDINATES:** N 523,237.56 E 2,306,433.29
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** G. Wilson **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 31 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Ash - West Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		(Fly ash)		SS -1	2.5-4.0	1-WH-1 (1)		
5				SS -2	4.5-6.0	2-2-4 (6)		
				SS -3	7.5-9.0	2-1-2 (3)		
10				UD -4	9.5-11.5			
15				SS -5	14.5-16.0	WH-WH-1 (1)		
			167.9					
20		Very soft, brownish red, CLAY (CL)		SS -6	19.5-21.0	2-1-1 (2)		
25				UD -7	24.5-26.5			

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Very soft, brownish red, CLAY (CL) *(Con't)*						
30		Stiff	156.4	SS -8	29.5-31.0	5-5-9 (14)		
		Bottom of borehole at 31.0 feet.						
35								
40								
45								
50								
55								

CONFIDENTIAL BUSINESS INFORMATION

SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/10/2010 **COMPLETED** 3/15/2010 **SURF. ELEV.** 194.8 **COORDINATES:** N 522,281.75 E 2,305,631.70
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger; Mud Rotary
DRILLED BY S. Denty **LOGGED BY** G. Wilson **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 70 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Crest - Northwest Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Stiff, Reddish tan sandy fat CLAY (CH)		SS -1	2.5-4.0	6-4-8 (12)		
5				SS -2	4.5-6.0	6-4-8 (12)		
		Reddish brown		SS -3	7.5-9.0	4-6-7 (13)		
			185.3					
10		Very stiff, brown, silty SAND (SM)		SS -4	9.5-11.0	5-9-9 (18)		
15				UD -5	14.5-16.5			c' = 130 psf Φ' = 32.8 deg. (MC = 18.6%; LL = 0; PL=NP; FC = 46.1%)
			178.3					
		Medium dense, reddish brown, clayey SAND (SC)						
20				SS -6	19.5-21.0	4-5-9 (14)		
25				SS -7	24.5-26.0	5-6-9 (15)		

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Medium dense, reddish brown, clayey SAND (SC) *(Con't)*						
30		Loose		SS -8	29.5-31.0	4-4-6 (10)		
35		Medium dense		SS -9	34.5-36.0	4-6-7 (13)		
40		Loose		SS -10	39.5-41.0	3-4-6 (10)		
45		Medium dense		SS -11	44.5-46.0	3-6-15 (21)		
			148.8					
		Medium dense, light tan, silty SAND (SM)						
50				UD -12	49.5-51.5			c' = 288 psf Φ' = 18.7 deg. (MC = 24.8%; LL = 0; PL=NP; FC = 47.7%)
55				SS -13	54.5-56.0	4-7-6 (13)		

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
60		Medium dense, light tan, silty SAND (SM) *(Con't)*		SS -14	59.5-61.0	7-9-11 (20)		
65								Due to material being pushed up into the augers, the drillers switched to mud rotary. Due to very little return of mud, the boring was terminated at approximately 70 feet..
70			124.8					
		Bottom of borehole at 70.0 feet.						
75								
80								
85								
90								

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/23/2010 **COMPLETED** 3/23/2010 **SURF. ELEV.** 172.1 **COORDINATES:** N 522,358.18 E 2,305,591.37
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** G. Wilson **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 36 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Toe - Northwest

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Loose, Reddish brown clayey SAND (SC)		SS -1	2.5-4.0	2-3-5 (8)		
5		Medium dense		SS -2	4.5-6.0	4-6-10 (16)		
				SS -3	7.5-9.0	5-5-7 (12)		
10				UD -4	9.5-11.5			
15				SS -5	14.5-16.0	5-7-9 (16)		
20				SS -6	19.5-21.0	6-7-9 (16)		
25				SS -7	24.5-26.0	3-5-6 (11)		CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
30		Loose		SS -8	29.5-31.0	4-3-5 (8)		
			137.6					
35		White, medium dense, clayey SAND/weathered limestone	136.1	SS -9	34.5-36.0	5-5-9 (14)		
		Bottom of borehole at 36.0 feet.						

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/16/2010 **COMPLETED** 3/16/2010 **SURF. ELEV.** 193.7 **COORDINATES:** N 521,792.93 E 2,305,413.89
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** J. Jordan **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 61 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Crest - West Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Stiff, brownish red, SANDY LEAN CLAY (CL)		SS -1	2.5-4.0	5-5-7 (12)		
5		Red, yellow and gray, mottled		SS -2	4.5-6.0	4-4-6 (10)		
				SS -3	7.5-9.0	5-5-8 (13)		
10				SS -4	9.5-11.0	4-5-7 (12)		
15				ST -5	14.5-16.5			c' = 100 psf Φ' = 33.4 deg. (MC = 19.7%; LL = 0; PL=NP; FC = 35.7%)
20		Firm, light gray to tan		SS -6	19.5-21.0	4-3-4 (7)		
25				SS -7	24.5-26.0	3-3-4 (7)		

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
30		Very stiff		SS -8	29.5-31.0	6-7-10 (17)		
35				SS -9	34.5-36.0	4-6-10 (16)		
40			154.2	SS -10	39.5-41.0	6-7-11 (18)		
		Medium dense, white, fine CLAYEY SAND (SC)						
45		Tan, cemented		SS -11	44.5-46.0	6-11-11 (22)		
50				ST -12	49.5-51.5			
55		Very loose, brown, wet		SS -13	54.5-56.0	WH-1-3 (4)		CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
60		White, medium dense, clayey sand / weathered limestone	134.2	SS -14	59.5-61.0	8-6-9 (15)		
			132.7					
		Bottom of borehole at 61.0 feet.						
65								
70								
75								
80								
85								
90								

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/22/2010 **COMPLETED** 3/22/2010 **SURF. ELEV.** 170.3 **COORDINATES:** N 521,793.48 E 2,305,320.86
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** G. Wilson **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 51 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Toe - West Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Stiff, reddish brown sandy CLAY (CL)						
				SS -1	2.5-4.0	4-5-6 (11)		
5		Very stiff		SS -2	4.5-6.0	6-8-11 (19)		
				SS -3	7.5-9.0	7-8-11 (19)		
10				UD -4	9.5-11.5			
			155.8					
15		Medium dense, brown, clayey SAND (SC)		SS -5	14.5-16.0	5-6-9 (15)		
20		Mottled gray/red/br		SS -6	19.5-21.0	5-4-6 (10)		
25		Loose		SS -7	24.5-26.0	4-4-5 (9)		CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Medium dense, brown, clayey SAND (SC) *(Con't)*						
30				SS -8	29.5-31.0	3-4-4 (8)		
35				SS -9	34.5-36.0	1-3-4 (7)		
40		Very loose		SS -10	39.5-41.0	3-2-1 (3)		
			125.8					
45		Very soft, brown sandy CLAY (CL)		SS -11	44.5-46.0	WR-WR-WR (0)		
			120.8					
50		White, medium dense, clayey sand / weathered limestone	119.3	SS -12	49.5-51.0	7-8-11 (19)		
		Bottom of borehole at 51.0 feet.						
55								

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/16/2010 **COMPLETED** 3/16/2010 **SURF. ELEV.** 194.2 **COORDINATES:** N 521,551.73 E 2,305,830.83
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** J. Jordan **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 56 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Crest - South Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Red and yellow, stiff SANDY LEAN CLAY (CL)		SS -1	2.5-4.0	5-6-9 (15)		
5		NO SAMPLE		SS -2	4.5-6.0	6-7-9 (16)		
		Very stiff, light gray to reddish yellow, mottled		SS -3	7.5-9.0	7-9-12 (21)		
10				SS -4	9.5-11.0	5-8-12 (20)		
15				ST -5	14.5-16.5			c' = 245 psf Φ' = 31.6 deg.
20		Medium dense, red and tan, mottled, fine CLAYEY SAND (SC)	174.7	SS -6	19.5-21.0	4-7-11 (18)		
25		Light brown		SS -7	24.5-26.0	5-7-10 (17)		CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Medium dense, red and tan, mottled, fine CLAYEY SAND (SC) *(Con't)*						
			164.7					
30		Very stiff, light brown, SANDY LEAN CLAY (CL)		SS -8	29.5-31.0	4-6-9 (15)		
			159.7					
35		Medium dense, gray, fine CLAYEY SAND (SC)		SS -9	34.5-36.0	4-7-11 (18)		
			154.7					
40		Gray, SANDY FAT CLAY (CH)		ST -10	39.5-41.5			c' = 260 psf Φ' = 23.6 deg.
45		Very stiff, dark red, yellow, and brown, with weathered limestone fragments		SS -11	44.5-46.0	5-7-10 (17)		
			144.7					
50		Medium dense, light gray to yellow, fine CLAYEY SAND (SC)		SS -12	49.5-51.0	4-5-9 (14)		
			139.7					
55		Stiff, grey FAT CLAY (CH) transitioning to white, sandy WEATHERED LIMESTONE	138.2	SS -13	54.5-56.0	5-6-5 (11)		

Bottom of borehole at 56.0 feet.

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DATE STARTED 3/17/2010 **COMPLETED** 3/17/2010 **SURF. ELEV.** 170.2 **COORDINATES:** N 521,454.45 E 2,305,828.28
CONTRACTOR SCS Field Services **EQUIPMENT** CME-550X **METHOD** Hollow Stem Auger
DRILLED BY S. Denty **LOGGED BY** J. Jordan **CHECKED BY** J. Jordan **ANGLE** _____ **BEARING** _____
BORING DEPTH 41 ft. **GROUND WATER DEPTH: DURING** _____ **COMP.** _____ **DELAYED** _____
NOTES Toe - South Side

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Soft, reddish brown, SANDY LEAN CLAY (CL)		SS -1	2.5-4.0	5-2-2 (4)		
5		Firm, layered, light orange to brown		SS -2	4.5-6.0	2-2-4 (6)		
		Gray with red and brown mottling		SS -3	7.5-9.0	4-6-8 (14)		
10				ST -4	9.5-11.5			c' = 200 psf Φ' = 30.7 deg. (MC = 16.7%; LL = 43; PI=24; FC = 51.1%)
15				SS -5	14.5-16.0	4-5-5 (10)		
20		Gray, red and brown, medium dense, fine CLAYEY SAND (SC)	150.7	SS -6	19.5-21.0	5-6-7 (13)		
25		Medium dense, wet, fine SAND (SP)	145.7	SS -7	24.5-26.0	6-6-9 (15)		

CONFIDENTIAL BUSINESS INFORMATION

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SOUTHERN COMPANY

LOG OF TEST BORING

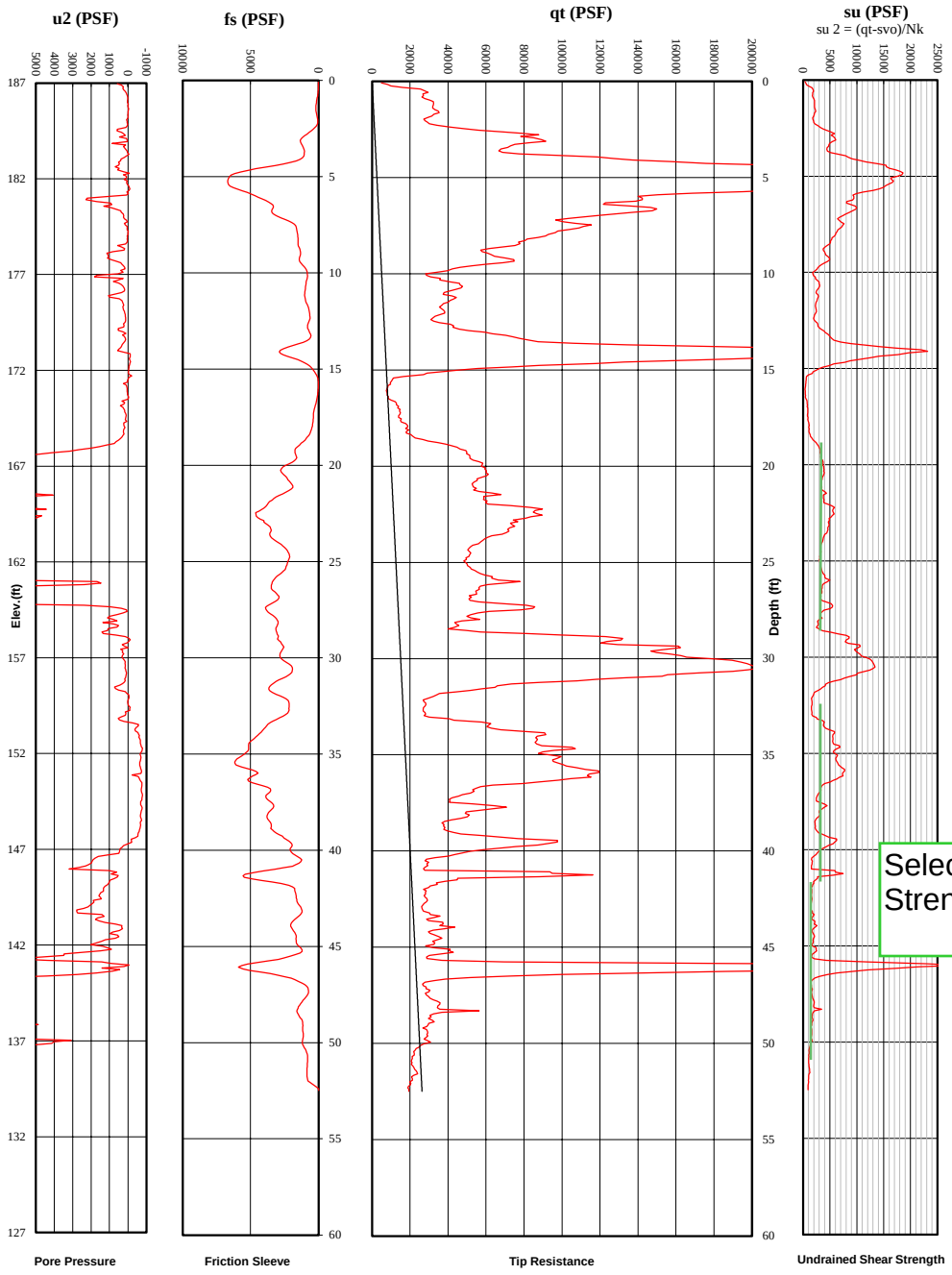
SOUTHERN COMPANY SERVICES, INC.
EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Mitchell Ash Ponds
LOCATION Albany, GA

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	SAMPLE TYPE NUMBER	SAMPLE DEPTH (ft.)	BLOW COUNTS (N VALUE)	RECOVERY % (RQD)	COMMENTS
		Medium dense, wet, fine SAND (SP) *(Con't)*						
30		Brown, increasing clay		SS -8	29.5-31.0	2-1-2 (3)		
			135.7					
35		Soft, grey to tan, SANDY FAT CLAY (CH)		SS -9	34.5-36.0	WH-WH-3 (3)		
			130.7					
40		White to yellow, sandy WEATHERED ROCK, effervescent in HCl	129.2	SS -10	39.5-41.0	WH-4-7 (11)		
		Bottom of borehole at 41.0 feet.						
45								
50								
55								

CONFIDENTIAL BUSINESS INFORMATION

GEOTECH ENGINEERING LOGS - ESEE DATABASE.GDT - 5/7/10 09:20 - T:\ESEE MAJOR PROJECTS\PROJECTS\MITCHELL STEAM PLANT\2010\ASH POND DIKE\LOGS.GPJ



INTERPRETATION

CCR - Very Soft to Firm Sandy Silt

Medium Dense Sand Seam

Firm Sandy Silt

Medium Dense Sand Seam

Soft

Coastal Terrace - Firm Silty Clay

Med. Dense Silty Sand Seam

Soft to Stiff Silty Clay

Selected Undrained Strength=1500-3000 psf

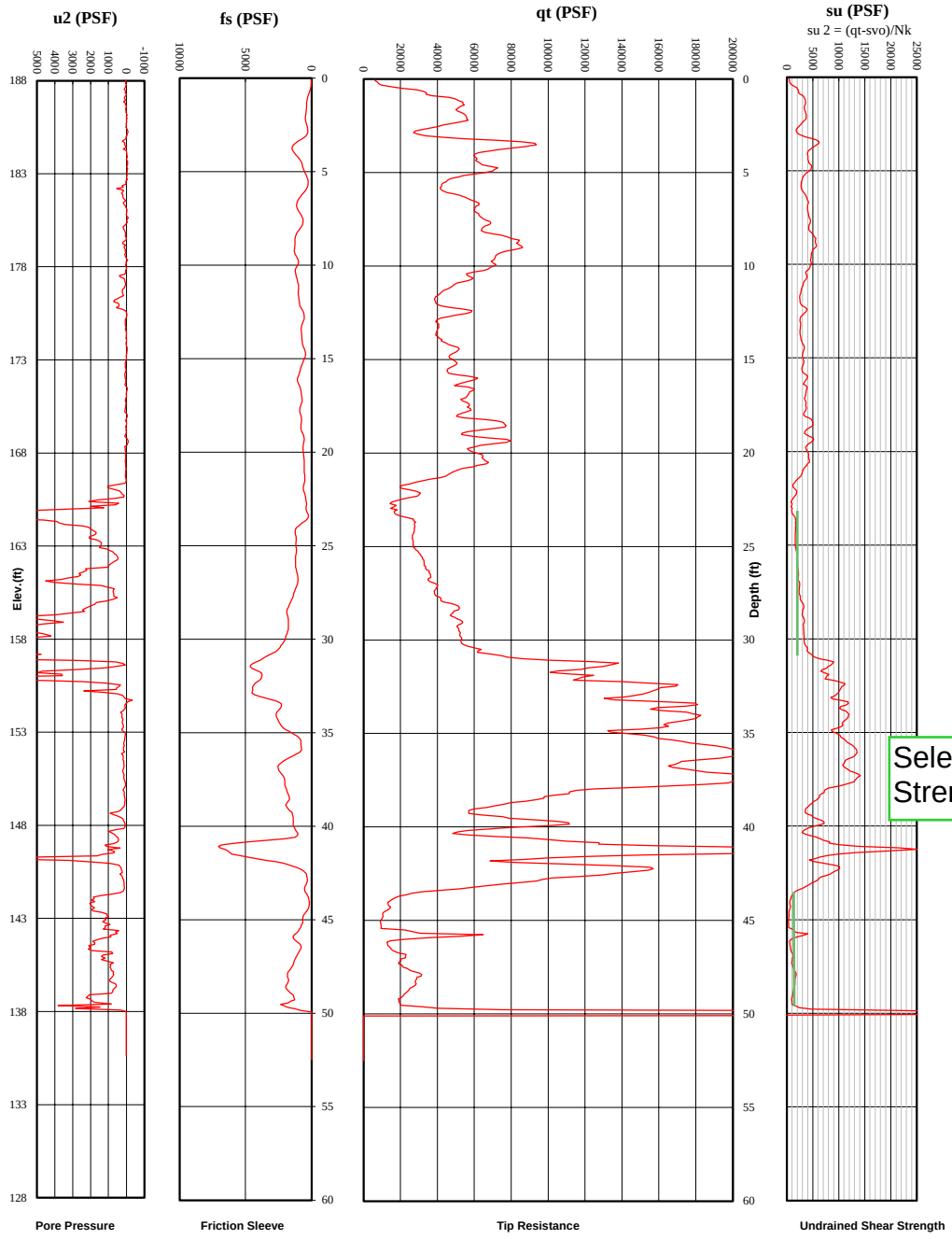
Very Soft Silty Clay

Probe Terminated @ 52.5'

wood.

Wood Environment and Infrastructure Solutions, Inc.
2677 Buford Highway, Atlanta, Georgia 30324
woodplc.com

PROJECT:	**Plant Mitchell Ash Pond Permit**
PROJ. NO:	**6121-17-0635**
PROBE ID:	**CPT-01**
DATE:	**2017-11-08**
NORTH:	**524384**
EAST:	**2306928**
SURFACE:	**187.2**



INTERPRETATION

CCR - Very Soft to Firm Sandy Silt

Soft

Coastal Terrace - Firm Silty Clay

Med. Dense Silty Sand

Selected Undrained Strength=1000-2000 psf

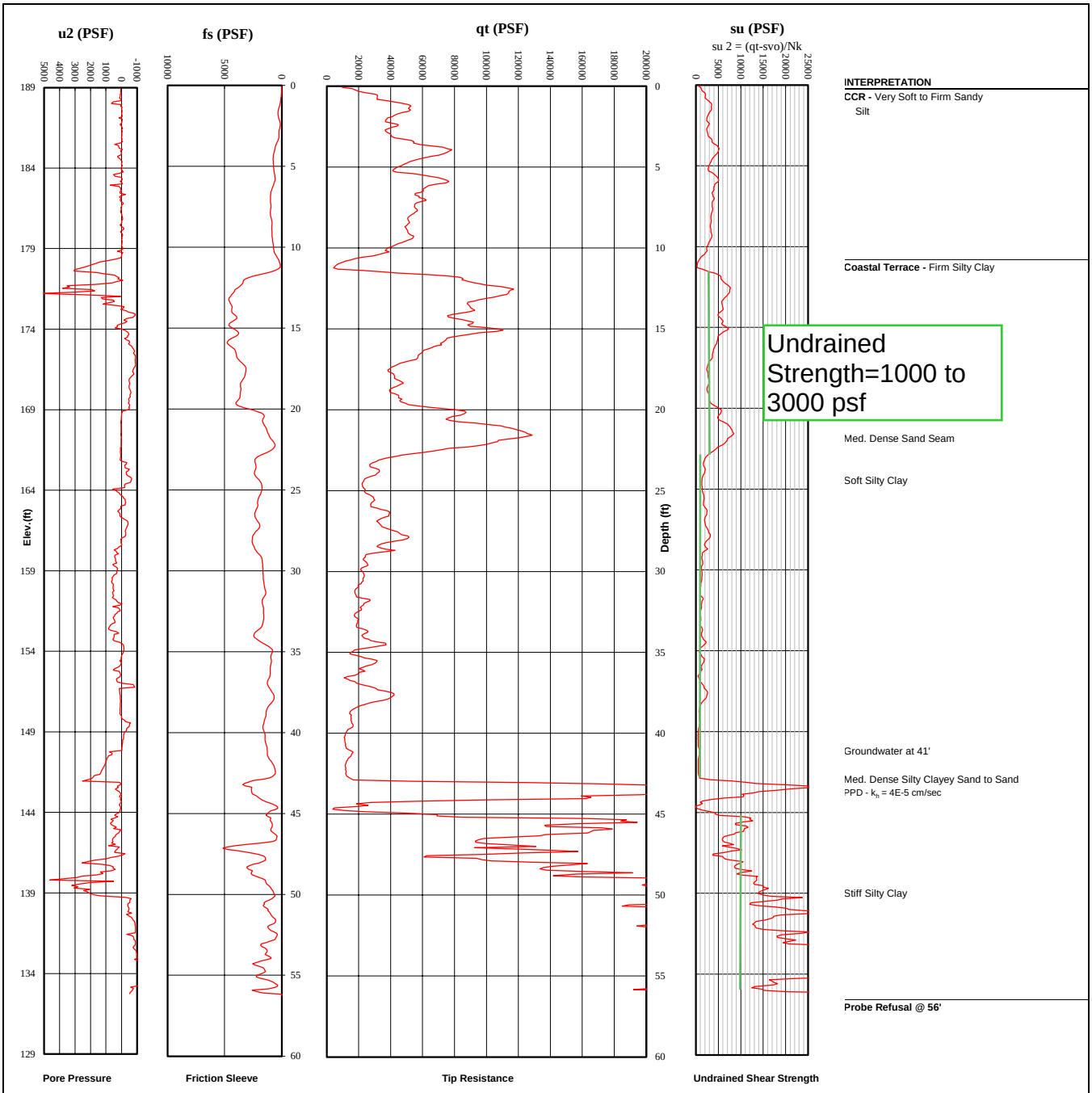
Very Soft Silty Clay

Probe Refusal @ 50'

wood.

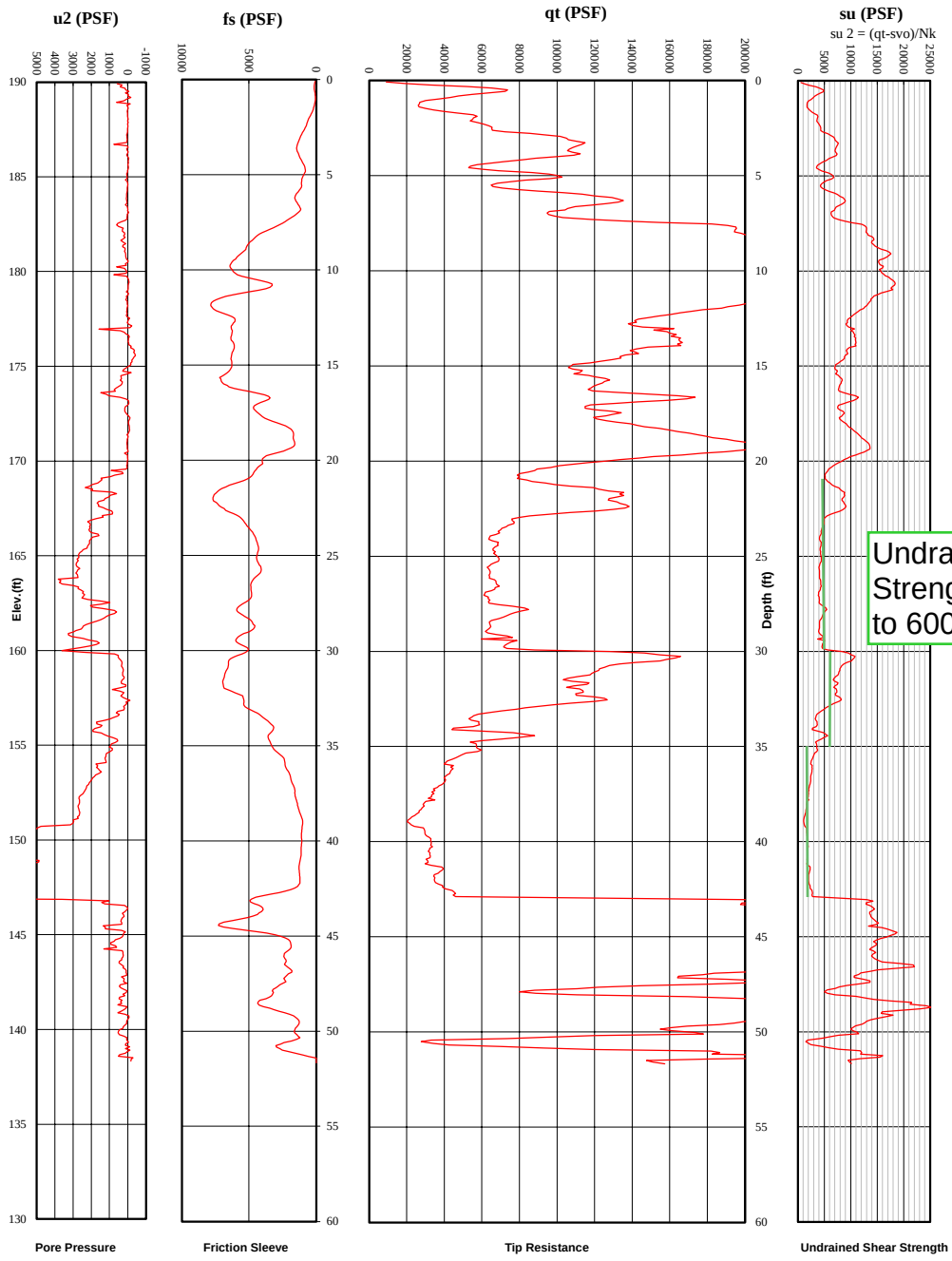
Wood Environment and Infrastructure Solutions, Inc.
2677 Buford Highway, Atlanta, Georgia 30324
woodplc.com

PROJECT:	**Plant Mitchell Ash Pond Permit**
PROJ. NO:	**6121-17-0635**
PROBE ID:	**CPT-02**
DATE:	**11/7/17**
NORTH:	**524125**
EAST:	**2306451**
SURFACE:	**188.4**



wood.
Wood Environment and Infrastructure Solutions, Inc.
2677 Buford Highway, Atlanta, Georgia 30324
woodplc.com

PROJECT:	**Plant Mitchell Ash Pond Permit**
PROJ. NO:	**6121-17-0635**
PROBE ID:	**CPT-03**
DATE:	**2017-11-07**
NORTH:	**523781**
EAST:	**2307415**
SURFACE:	**188.6**



INTERPRETATION

CCR - Loose to Medium Dense Silty Sand w/Silt Seams

Coastal Terrace - Firm Silty Clay

Undrained Strength=1500 psf to 6000 psf

Med. Dense Sandy Clay

Soft Silty Clay

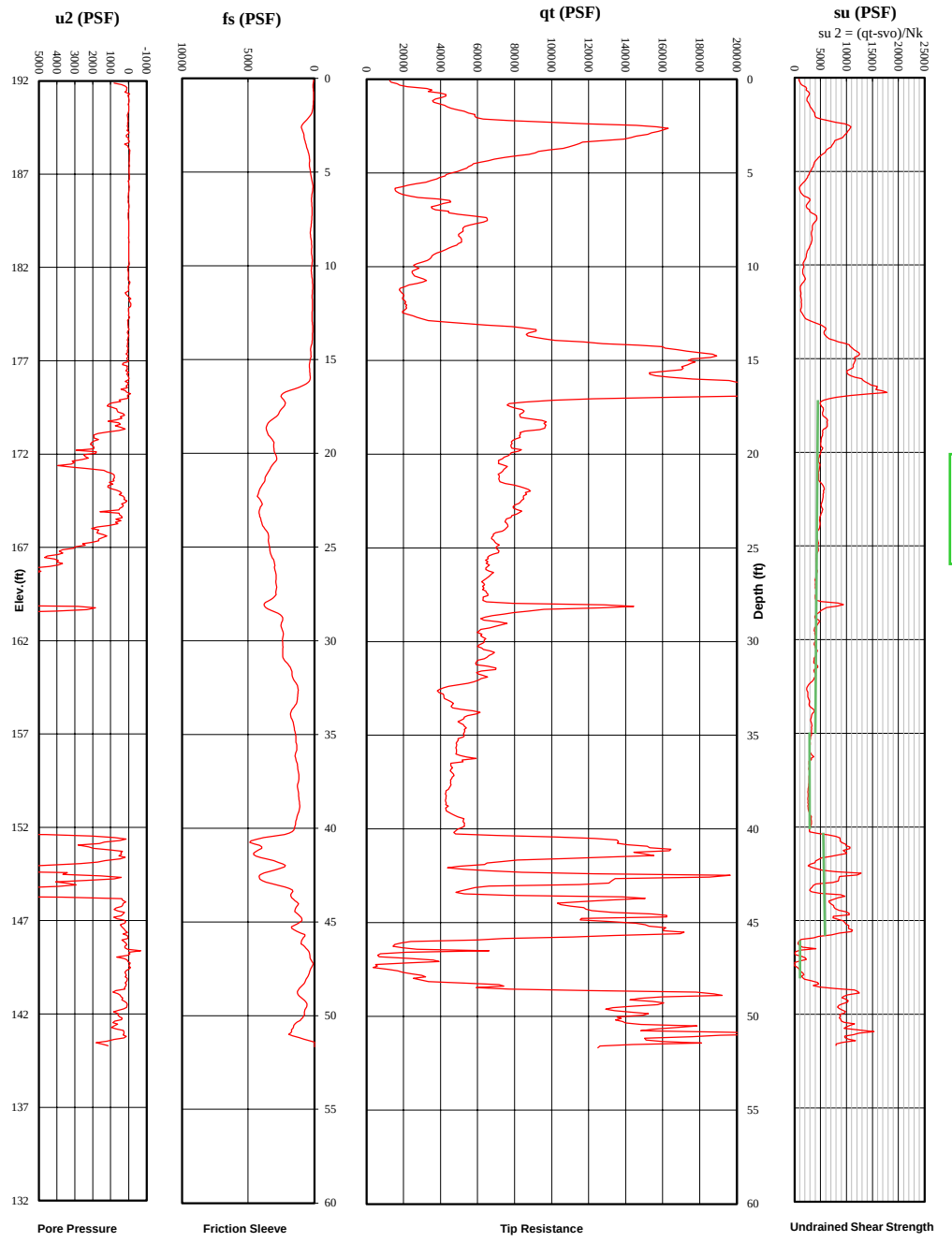
Med. Dense Silty Clayey Sand to Sand

Probe Terminated @ 51.7'

wood.

Wood Environment and Infrastructure Solutions, Inc.
2677 Buford Highway, Atlanta, Georgia 30324
woodplc.com

PROJECT:	**Plant Mitchell Ash Pond Permit**
PROJ. NO:	**6121-17-0635**
PROBE ID:	**CPT-04**
DATE:	**11/8/17**
NORTH:	**523117**
EAST:	**2306880**
SURFACE:	**190.4**



INTERPRETATION

CCR - Soft to Firm Sandy Silt

Med. Dense Sand Seam

Soft to Firm Sandy Silt

Medium Dense Sand

Coastal Terrace - Firm Silty Clay

Undrained Strength=1000 psf to 6000 psf

Sand Seam

Soft Silty Clay

Med. Dense Silty Clayey Sand to Sand

Soft Clay

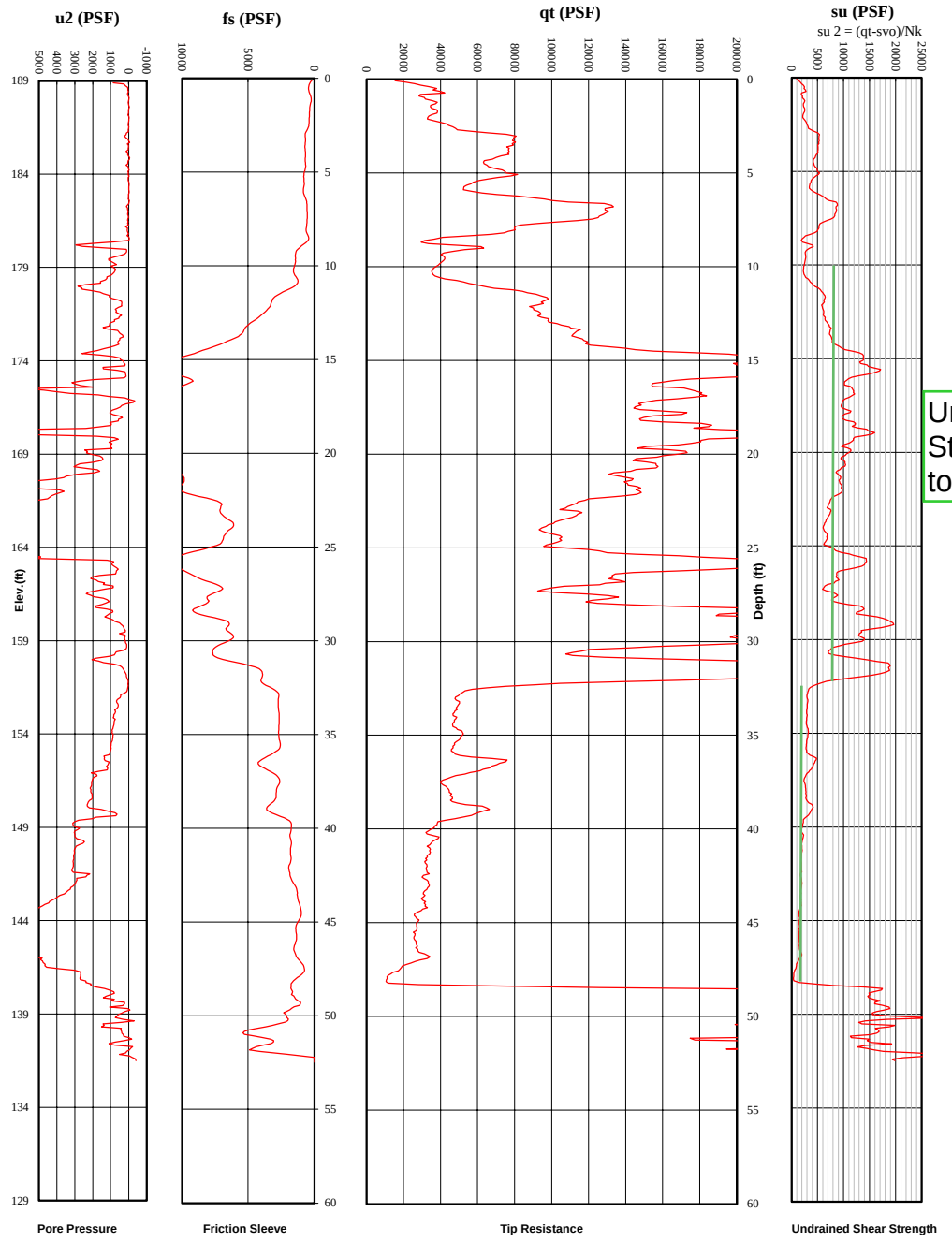
Med. Dense Silty Sand

Probe Terminated @ 51.7'

wood.

Wood Environment and Infrastructure Solutions, Inc.
2677 Buford Highway, Atlanta, Georgia 30324
woodplc.com

PROJECT:	**Plant Mitchell Ash Pond Permit**
PROJ. NO:	**6121-17-0635**
PROBE ID:	**CPT-05**
DATE:	**2017-11-08**
NORTH:	**522700**
EAST:	**2307083**
SURFACE:	**192.3**



INTERPRETATION

CCR - Soft to Firm Sandy Silt

Stiff

Soft

Coastal Terrace - Loose to Medium Dense
Clayey Silty Sand

Undrained Strength=2000 psf to 8000 psf

Soft Silty Clay

Groundwater at 43'

Med. Dense Clayey Silty Sand
PPD - k_h=2E-5 cm/sec

Probe Terminated @ 52.5'

wood.
Wood Environment and Infrastructure Solutions, Inc.
2677 Buford Highway, Atlanta, Georgia 30324
woodplc.com

PROJECT:	**Plant Mitchell Ash Pond Permit**
PROJ. NO:	**6121-17-0635**
PROBE ID:	**CPT-10**
DATE:	**11/8/17**
NORTH:	**523864**
EAST:	**2306956**
SURFACE:	**188.7**

Table 3
Summary of Groundwater Elevations
Georgia Power Company
Plant Mitchell - 1365.11|3.04|0.2
Dougherty County, US-GA

					August 2016		December 2016		March 2017		July 2017		October 2017	
Overburden/Bedrock	**Hydraulic Location**	**AP-1 Wells**	**Well ID**	**Top of Casing Elevation (feet)**	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)
Overburden	Downgradient	AP-1	MW-101	170.93										
Bedrock	Downgradient	AP-1	MW-102	170.93	34.46	136.47	31.84	139.09	30.31	140.62	30.56	140.37	33.38	137.55
Bedrock	Downgradient		MW-105	187.52	49.88	137.64	48.35	139.17	45.35	142.17	46.59	140.93	48.62	138.90
Bedrock	Downgradient		MW-108	185.47	48.76	136.71	47.27	138.20	44.72	140.75	45.38	140.09	47.57	137.90
Bedrock	Downgradient		MW-111	168.06	32.63	135.43	30.07	137.99	28.58	139.48	29.13	138.93	30.63	137.43
Bedrock	Downgradient		MW-113	174.61	38.33	136.28	35.71	138.90	33.97	140.64	34.72	139.89	37.03	137.58
Bedrock	Downgradient		MW-115	169.05	33.22	135.83	30.65	138.40	28.87	140.18	29.62	139.43	31.27	137.78
Bedrock	Downgradient	AP-1	MW-116	171.69	35.87	135.82	33.16	138.53	31.80	139.89	31.96	139.73	33.78	137.91
Overburden	Downgradient		MW-119	194.49										
Bedrock	Downgradient		MW-120	193.79										
Bedrock	Downgradient	AP-1	PZ-01R	191.87			52.95	138.92	51.27	140.60	51.85	140.02	53.62	138.25
Bedrock	Downgradient		PZ-02R	191.66			52.61	139.05	50.55	141.11	51.24	140.42	53.24	138.42
Bedrock	Downgradient		PZ-03R	192.35			53.74	138.61	51.88	140.47	52.26	140.09	54.34	138.01
Bedrock	Upgradient		PZ-1D	196.44	55.93	140.51	52.68	143.76	50.83	145.61	52.86	143.58	55.09	141.35
Overburden	Upgradient		PZ-1S	196.52										
Bedrock	Upgradient		PZ-2D	178.51	39.60	138.91	35.30	143.21	34.45	144.06	36.07	142.44	38.28	140.23
Overburden	Upgradient		PZ-2S	178.61	39.64	138.97	35.31	143.30	34.47	144.14	36.12	142.49	38.35	140.26
Bedrock	Upgradient		PZ-3D	190.98	52.44	138.54	49.69	141.29	47.52	143.46	48.92	142.06	50.95	140.03
Overburden	Upgradient		PZ-3S	191.12										
Bedrock	Downgradient	AP-1	PZ-4D	191.10	52.83	138.27	52.18	138.92	48.34	142.76	49.86	141.24	52.04	139.06
Overburden	Downgradient	AP-1	PZ-4S	191.20										
Bedrock	Downgradient		PZ-5D	193.82	54.94	138.88	55.49	138.33	50.65	143.17	52.76	141.06	54.76	139.06
Overburden	Downgradient		PZ-6S	189.47	27.64	161.83	13.85	175.62	15.77	173.70	24.45	165.02	31.76	157.71
Bedrock	Downgradient		PZ-7D	173.08	37.26	135.82	34.33	138.75	33.16	139.92	33.11	139.97	35.13	137.95
Overburden	Downgradient		PZ-7S	173.10										
Bedrock	Downgradient		PZ-8D	170.35	34.94	135.41	32.22	138.13	30.84	139.51	31.47	138.88	33.01	137.34
Overburden	Downgradient		PZ-8S	170.78										
Bedrock	Downgradient		PZ-9D	166.16	30.77	135.39	28.09	138.07	26.67	139.49	27.10	139.06	28.65	137.51
Overburden	Downgradient		PZ-9S	166.02										
Bedrock	Downgradient		PZ-10S	175.63	39.74	135.89	36.79	138.84	35.44	140.19	35.86	139.77	38.04	137.59
Bedrock	Downgradient	AP-1	PZ-11S	191.69	56.01	135.68	53.18	138.51	52.69	139.00	51.99	139.70	53.90	137.79
Bedrock	Downgradient	AP-1	PZ-12S	173.92	38.23	135.69	35.60	138.32	34.04	139.88	34.53	139.39	36.24	137.68
Overburden	Downgradient	AP-1	PZ-13S	173.22										
Bedrock	Downgradient		PZ-14	183.46	46.98	136.48	45.36	138.10	42.71	140.75	43.53	139.93	45.70	137.76
Bedrock	Downgradient		PZ-15	170.37	34.92	135.45	32.35	138.02	30.75	139.62	31.27	139.10	32.74	137.63
Bedrock	Downgradient		PZ-16	173.92	38.21	135.71	35.10	138.82	33.86	140.06	34.25	139.67	36.17	137.75
Bedrock	Downgradient		PZ-17	172.91	36.65	136.26	34.10	138.81	32.45	140.46	32.86	140.05	34.86	138.05
Bedrock	Downgradient	AP-1	PZ-18	170.11	34.06	136.05	31.48	138.63	29.80	140.31	30.25	139.86	32.02	138.09
Bedrock	Downgradient	AP-1	PZ-19	172.05	35.95	136.10	33.21	138.84	31.75	140.30	31.98	140.07	34.02	138.03
Bedrock	Downgradient	AP-1	PZ-20	173.44	37.20	136.24	34.66	138.78	33.16	140.28	33.29	140.15	35.24	138.20
Bedrock	Downgradient	AP-1	PZ-21	179.84	42.67	137.17	40.24	139.60	38.11	141.73	38.96	140.88	40.89	138.95
Bedrock	Downgradient	AP-1	PZ-22	187.69	49.84	137.85	47.74	139.95	45.02	142.67	46.54	141.15	48.40	139.29
Bedrock	Downgradient	AP-1	PZ-23	191.62	53.49	138.13	52.97	138.65	49.07	142.55	50.76	140.86	52.84	138.78
Bedrock	Downgradient	AP-1	PZ-23A	191.85										
Bedrock	Downgradient	AP-1	PZ-24	194.91	57.70	137.21	56.68	138.23	53.41	141.50	54.65	140.26	56.78	138.13
Bedrock	Downgradient	AP-1	PZ-24A	194.97										
Bedrock	Downgradient	AP-1	PZ-25	171.14	34.66	136.48	32.02	139.12	30.51	140.63	30.71	140.43	32.60	138.54
Bedrock	Downgradient		PZ-26	166.70			28.72	137.98	27.22	139.48	27.85	138.85	29.36	137.34
Bedrock	Downgradient		PZ-27	164.58			25.78	138.80	24.58	140.00	24.98	139.60	26.96	137.62
Bedrock	Downgradient	AP-1	PZ-28	165.96			26.92	139.04	25.76	140.20	26.18	139.78	28.26	137.70
Bedrock	Downgradient	AP-1	PZ-29	173.18			34.86	138.32	33.30	139.88	33.83	139.35	35.38	137.80
Bedrock	Upgradient		PZ-31	182.96			39.82	143.14	37.01	145.95	39.50	143.46	41.69	141.27
Bedrock	Upgradient		PZ-32	180.75			36.98	143.77	36.44	144.31	38.13	142.62	40.39	140.36
Bedrock	Downgradient		PZ-33	189.61			50.80	138.81	48.40	141.21	49.22	140.39	51.19	138.42
Bedrock	Downgradient	AP-1	PZ-57	169.35										

Table 3
Summary of Groundwater Elevations
Georgia Power Company
Plant Mitchell - 1365.11|3.04|0.2
Dougherty County, US-GA

				February 2018		July 2018		September 2018		March 2019		August 2019		October 2019	
Overburden/Bedrock	**Hydraulic Location**	**AP-1 Wells**	**Well ID**	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)
Overburden	Downgradient	AP-1	MW-101												
Bedrock	Downgradient	AP-1	MW-102	27.34	143.59	31.56	139.37	32.58	138.35	28.96	141.97	32.65	138.28	34.38	136.55
Bedrock	Downgradient		MW-105	44.54	142.98	47.40	140.12	47.60	139.92	42.93	144.59	48.19	139.33		
Bedrock	Downgradient		MW-108	43.16	142.31	46.17	139.30	46.70	138.77	42.86	142.61	47.62	137.85	48.69	136.78
Bedrock	Downgradient		MW-111	26.17	141.89	29.62	138.44	31.26	136.80	27.22	140.84	30.63	137.43	32.60	135.46
Bedrock	Downgradient		MW-113	31.06	143.55	35.87	138.74	36.42	138.19	31.40	143.21	37.42	137.19	38.33	136.28
Bedrock	Downgradient		MW-115	26.62	142.43	30.15	138.90	31.67	137.38	27.51	141.54	31.15	137.90	33.11	135.94
Bedrock	Downgradient	AP-1	MW-116	29.19	142.50	32.98	138.71	34.14	137.55	30.25	141.44	34.21	137.48	35.72	135.97
Overburden	Downgradient		MW-119												
Bedrock	Downgradient		MW-120												
Bedrock	Downgradient	AP-1	PZ-01R	48.98	142.89	52.86	139.01	53.79	138.08	49.73	142.14	53.39	138.48	55.54	136.33
Bedrock	Downgradient		PZ-02R	48.89	142.77	52.34	139.32	53.30	138.36	48.83	142.83	53.34	138.32	54.78	136.88
Bedrock	Downgradient		PZ-03R	49.32	143.03	53.57	138.78	54.55	137.80	50.61	141.74	54.71	137.64	56.00	136.35
Bedrock	Upgradient		PZ-1D	50.39	146.05	53.39	143.05	52.09	144.35	48.29	148.15	53.98	142.46	55.95	140.49
Overburden	Upgradient		PZ-1S												
Bedrock	Upgradient		PZ-2D	32.59	145.92	36.48	142.03	35.81	142.70	31.68	146.83	37.15	141.36	39.42	139.09
Overburden	Upgradient		PZ-2S	32.65	145.96	36.56	142.05	35.85	142.76	31.72	146.89	37.25	141.36	39.45	139.16
Bedrock	Upgradient		PZ-3D	45.69	145.29	49.46	141.52	48.98	142.00	44.90	146.08	50.27	140.71	52.24	138.74
Overburden	Upgradient		PZ-3S												
Bedrock	Downgradient	AP-1	PZ-4D	49.11	141.99	50.46	140.64	50.51	140.59	45.75	145.35	51.16	139.94	52.49	138.61
Overburden	Downgradient	AP-1	PZ-4S												
Bedrock	Downgradient		PZ-5D	53.32	140.50	53.01	140.81	52.72	141.10	47.83	145.99	52.99	140.83		
Overburden	Downgradient		PZ-6S	17.41	172.06	21.92	167.55	16.43	173.04	15.23	174.24	20.23	169.24	24.87	164.60
Bedrock	Downgradient		PZ-7D	30.00	143.08	33.61	139.47	35.33	137.75	31.53	141.55	35.34	137.74	37.15	135.93
Overburden	Downgradient		PZ-7S												
Bedrock	Downgradient		PZ-8D	28.48	141.87	32.08	138.27	33.55	136.80	29.52	140.83	32.95	137.40	34.89	135.46
Overburden	Downgradient		PZ-8S												
Bedrock	Downgradient		PZ-9D	23.90	142.26	27.78	138.38	28.98	137.18	25.36	140.80	28.86	137.30	30.66	135.50
Overburden	Downgradient		PZ-9S												
Bedrock	Downgradient		PZ-10S	32.33	143.30	36.97	138.66	37.81	137.82	33.72	141.91	38.42	137.21	39.61	136.02
Bedrock	Downgradient	AP-1	PZ-11S	48.70	142.99	53.08	138.61	54.18	137.51	50.28	141.41	54.51	137.18	55.89	135.80
Bedrock	Downgradient	AP-1	PZ-12S	31.30	142.62	35.34	138.58	36.41	137.51	32.68	141.24	36.26	137.66	38.13	135.79
Overburden	Downgradient	AP-1	PZ-13S												
Bedrock	Downgradient		PZ-14	41.21	142.25	44.38	139.08	44.84	138.62	40.93	142.53	45.58	137.88	46.72	136.74
Bedrock	Downgradient		PZ-15	28.38	141.99	31.69	138.68	33.44	136.93	29.54	140.83	32.88	137.49	34.87	135.50
Bedrock	Downgradient		PZ-16	30.78	143.14	35.36	138.56	36.28	137.64	32.32	141.60	36.81	137.11	38.13	135.79
Bedrock	Downgradient		PZ-17	29.62	143.29	33.80	139.11	34.82	138.09	30.50	142.41	34.82	138.09	36.65	136.26
Bedrock	Downgradient	AP-1	PZ-18	27.17	142.94	31.17	138.94	32.11	138.00	27.88	142.23	32.19	137.92	33.97	136.14
Bedrock	Downgradient	AP-1	PZ-19	28.82	143.23	33.24	138.81	34.22	137.83	30.11	141.94	34.52	137.53	35.91	136.14
Bedrock	Downgradient	AP-1	PZ-20	30.33	143.11	34.51	138.93	35.59	137.85	31.68	141.76	35.87	137.57	37.15	136.29
Bedrock	Downgradient	AP-1	PZ-21	35.82	144.02	39.81	140.03	40.40	139.44	36.10	143.74	41.04	138.80	42.61	137.23
Bedrock	Downgradient	AP-1	PZ-22	43.25	144.44	47.15	140.54	46.99	140.70	42.52	145.17	48.25	139.44	49.63	138.06
Bedrock	Downgradient	AP-1	PZ-23	50.07	141.55	51.33	140.29	51.50	140.12	46.32	145.30	52.84	138.78		
Bedrock	Downgradient	AP-1	PZ-23A												
Bedrock	Downgradient	AP-1	PZ-24	53.20	141.71	55.45	139.46	55.91	139.00	51.10	143.81	56.26	138.65		
Bedrock	Downgradient	AP-1	PZ-24A												
Bedrock	Downgradient	AP-1	PZ-25	27.51	143.63	31.81	139.33	32.80	138.34	29.19	141.95	32.89	138.25	34.66	136.48
Bedrock	Downgradient		PZ-26	24.83	141.87	28.43	138.27	29.94	136.76	25.90	140.80	29.28	137.42	31.26	135.44
Bedrock	Downgradient		PZ-27	21.82	142.76	26.10	138.48	27.16	137.42	23.34	141.24	27.47	137.11	28.83	135.75
Bedrock	Downgradient	AP-1	PZ-28	23.15	142.81	27.26	138.70	28.11	137.85	24.55	141.41	28.83	137.13	29.73	136.23
Bedrock	Downgradient	AP-1	PZ-29	30.60	142.58	34.63	138.55	35.61	137.57	32.02	141.16	35.44	137.74	37.41	135.77
Bedrock	Upgradient		PZ-31	37.57	145.39	39.61	143.35	38.79	144.17	34.67	148.29	40.73	142.23	42.56	140.40
Bedrock	Upgradient		PZ-32	34.66	146.09	38.83	141.92	37.61	143.14	33.72	147.03	39.64	141.11	41.57	139.18
Bedrock	Downgradient		PZ-33	46.59	143.02	50.23	139.38	50.87	138.74	46.68	142.93	51.23	138.38	52.54	137.07
Bedrock	Downgradient	AP-1	PZ-57												

Table 3
Summary of Groundwater Elevations
Georgia Power Company
Plant Mitchell - 1365.11|3.04|0.2
Dougherty County, US-GA

				March 2020		August 2020		October 2020		March 2021		September 2021		January 2022	
Overburden/Bedrock	**Hydraulic Location**	**AP-1 Wells**	**Well ID**	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)
Overburden	Downgradient	AP-1	MW-101												
Bedrock	Downgradient	AP-1	MW-102	22.37	148.56	30.31	140.62	29.50	141.43	24.54	146.39	31.61	139.32	26.95	143.98
Bedrock	Downgradient		MW-105												
Bedrock	Downgradient		MW-108	37.71	147.76	46.35	139.12	43.91	141.56	38.71	146.76	45.76	139.71	41.28	144.19
Bedrock	Downgradient		MW-111	22.95	145.11	28.23	139.83	28.18	139.88	23.64	144.42	30.15	137.91	25.54	142.52
Bedrock	Downgradient		MW-113	22.65	151.96	35.66	138.95	33.37	141.24	27.43	147.18	34.40	140.21	30.15	144.46
Bedrock	Downgradient		MW-115	22.70	146.35	28.92	140.13	31.28	137.77	24.05	145.00	30.46	138.59		
Bedrock	Downgradient	AP-1	MW-116	24.17	147.52	32.13	139.56	30.87	140.82	26.10	145.59	32.85	138.84	28.33	143.36
Overburden	Downgradient		MW-119												
Bedrock	Downgradient		MW-120												
Bedrock	Downgradient	AP-1	PZ-01R									52.61	139.26	47.95	143.92
Bedrock	Downgradient		PZ-02R									52.05	139.61	47.66	144.00
Bedrock	Downgradient		PZ-03R												
Bedrock	Upgradient		PZ-1D	40.42	156.02	52.98	143.46	50.46	145.98	41.17	155.27	52.29	144.15	47.97	148.47
Overburden	Upgradient		PZ-1S												
Bedrock	Upgradient		PZ-2D	22.64	155.87	36.10	142.41	33.85	144.66	23.50	155.01	35.67	142.84	31.17	147.34
Overburden	Upgradient		PZ-2S	22.55	156.06	36.21	142.40	33.90	144.71	23.56	155.05	35.71	142.90	31.24	147.37
Bedrock	Upgradient		PZ-3D	37.44	153.54	48.89	142.09	46.66	144.32	38.26	152.72	48.49	142.49	44.03	146.95
Overburden	Upgradient		PZ-3S												
Bedrock	Downgradient	AP-1	PZ-4D	39.61	151.49	50.41	140.69	47.47	143.63	41.34	149.76	49.17	141.93	46.07	145.03
Overburden	Downgradient	AP-1	PZ-4S												
Bedrock	Downgradient		PZ-5D												
Overburden	Downgradient		PZ-6S	11.22	178.25	18.96	170.51	16.17	173.30	10.72	178.75	19.91	169.56	17.90	171.57
Bedrock	Downgradient		PZ-7D	27.04	146.04	33.28	139.80	32.28	140.80	27.39	145.69	34.15	138.93	29.63	143.45
Overburden	Downgradient		PZ-7S												
Bedrock	Downgradient		PZ-8D	25.03	145.32	30.32	140.03	30.22	140.13	25.82	144.53	32.33	138.02	27.76	142.59
Overburden	Downgradient		PZ-8S												
Bedrock	Downgradient		PZ-9D	20.94	145.22	26.67	139.49	26.01	140.15	21.70	144.46	27.94	138.22	23.54	142.62
Overburden	Downgradient		PZ-9S												
Bedrock	Downgradient		PZ-10S	27.48	148.15	36.61	139.02	34.33	141.30	29.57	146.06	36.45	139.18	32.20	143.43
Bedrock	Downgradient	AP-1	PZ-11S	45.63	146.06	52.61	139.08	50.78	140.91	46.66	145.03	52.75	138.94	48.36	143.33
Bedrock	Downgradient	AP-1	PZ-12S	27.16	146.76	34.05	139.87	33.58	140.34	28.97	144.95	35.64	138.28	30.60	143.32
Overburden	Downgradient	AP-1	PZ-13S												
Bedrock	Downgradient		PZ-14	36.02	147.44	44.23	139.23	42.00	141.46	36.89	146.57	43.80	139.66	39.32	144.14
Bedrock	Downgradient		PZ-15	25.21	145.16	30.52	139.85	30.17	140.20	26.16	144.21	32.26	138.11	27.59	142.78
Bedrock	Downgradient		PZ-16	27.27	146.65	35.02	138.90	32.98	140.94	28.55	145.37	35.01	138.91	30.48	143.44
Bedrock	Downgradient		PZ-17	25.21	147.70	32.07	140.84	31.52	141.39	27.02	145.89	33.63	139.28	29.13	143.78
Bedrock	Downgradient	AP-1	PZ-18	22.71	147.40	30.04	140.07	28.78	141.33	24.41	145.70	30.74	139.37	26.34	143.77
Bedrock	Downgradient	AP-1	PZ-19	24.16	147.89	32.56	139.49	30.92	141.13	26.14	145.91	32.92	139.13	28.54	143.51
Bedrock	Downgradient	AP-1	PZ-20	24.60	148.84	33.74	139.70	32.35	141.09	27.46	145.98	34.30	139.14	30.03	143.41
Bedrock	Downgradient	AP-1	PZ-21	30.71	149.13	39.09	140.75	37.20	142.64	31.83	148.01	39.24	140.60	34.70	145.14
Bedrock	Downgradient	AP-1	PZ-22	36.71	150.98	46.78	140.91	44.06	143.63	37.81	149.88	46.04	141.65	41.56	146.13
Bedrock	Downgradient	AP-1	PZ-23												
Bedrock	Downgradient	AP-1	PZ-23A	40.11	151.74	50.27	141.58	48.23	143.62	42.69	149.16	49.25	142.60	47.32	144.53
Bedrock	Downgradient	AP-1	PZ-24												
Bedrock	Downgradient	AP-1	PZ-24A	45.26	149.71	54.74	140.23	52.42	142.55	47.17	147.80	54.21	140.76	50.61	144.36
Bedrock	Downgradient	AP-1	PZ-25	22.41	148.73	30.57	140.57	29.74	141.40	24.70	146.44	31.82	139.32	27.33	143.81
Bedrock	Downgradient		PZ-26	21.63	145.07	27.03	139.67	26.68	140.02	22.30	144.40				
Bedrock	Downgradient		PZ-27	16.88	147.70	25.68	138.90	23.81	140.77	18.90	145.68	25.91	138.67	21.54	143.04
Bedrock	Downgradient	AP-1	PZ-28	19.03	146.93	26.98	138.98	24.97	140.99	20.68	145.28	26.88	139.08	22.79	143.17
Bedrock	Downgradient	AP-1	PZ-29	26.91	146.27	33.04	140.14	33.01	140.17	28.23	144.95	35.03	138.15	29.93	143.25
Bedrock	Upgradient		PZ-31	26.84	156.12	39.83	143.13	37.15	145.81	28.89	154.07	38.84	144.12	35.35	147.61
Bedrock	Upgradient		PZ-32	25.02	155.73	38.48	142.27	35.83	144.92	24.99	155.76	37.78	142.97	32.96	147.79
Bedrock	Downgradient		PZ-33	41.40	148.21	49.63	139.98	47.67	141.94	42.52	147.09	49.55	140.06	45.58	144.03
Bedrock	Downgradient	AP-1	PZ-57											25.78	143.57

Table 3
Summary of Groundwater Elevations
Georgia Power Company
Plant Mitchell - 1365.11|3.04|0.2
Dougherty County, US-GA

				August 2022		February 2023		September 2023		February 2024		August 2024		March 2025		September 2025	
Overburden/Bedrock	Hydraulic Location	AP-1 Wells	Well ID	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)	Depth to Water (feet)	Groundwater Elevation (feet)
Overburden	Downgradient	AP-1	MW-101	23.16	147.77	10.32	160.61	20.51	150.42	14.64	156.29	17.95	152.98	10.77	160.16	15.78	155.15
Bedrock	Downgradient	AP-1	MW-102	32.70	138.23	23.68	147.25	33.28	137.65	25.95	144.98	31.68	139.25	27.68	143.25	31.40	139.53
Bedrock	Downgradient		MW-105														
Bedrock	Downgradient		MW-108														
Bedrock	Downgradient		MW-111	30.36	137.70	22.02	146.04	31.48	136.58	25.17	142.89	30.60	137.46	25.86	142.20		
Bedrock	Downgradient		MW-113	36.97	137.64	28.04	146.57	37.23	137.38	28.80	145.81	33.68	140.93	31.82	142.79		
Bedrock	Downgradient		MW-115														
Bedrock	Downgradient	AP-1	MW-116	34.09	137.60	25.16	146.53	34.65	137.04	27.19	144.50	32.72	138.97	29.54	142.15	32.29	139.40
Overburden	Downgradient		MW-119	38.46	156.03	37.63	156.86										
Bedrock	Downgradient		MW-120			46.15	147.64			45.69	148.10						
Bedrock	Downgradient	AP-1	PZ-01R	54.09	137.78	45.47	146.40	54.41	137.46	47.39	144.48	52.42	139.45	49.35	142.52		
Bedrock	Downgradient		PZ-02R	54.12	137.54	45.92	145.74										
Bedrock	Downgradient		PZ-03R														
Bedrock	Upgradient		PZ-1D	55.16	141.28	46.33	150.11	55.13	141.31	46.31	150.13	51.72	144.72	50.61	145.83	52.93	143.51
Overburden	Upgradient		PZ-1S	41.54	154.98	38.95	157.57	37.94	158.58	35.93	160.59	34.93	161.59	38.70	157.82	37.83	158.69
Bedrock	Upgradient		PZ-2D	38.62	139.89	28.38	150.13	38.66	139.85	29.80	148.71	35.27	143.24	33.95	144.56	36.29	142.22
Overburden	Upgradient		PZ-2S	38.69	139.92	28.53	150.08	38.74	139.87	29.88	148.73	35.28	143.33	34.05	144.56	36.34	142.27
Bedrock	Upgradient		PZ-3D	51.40	139.58	41.62	149.36	51.34	139.64	42.59	148.39	48.08	142.90	46.48	144.50	48.92	142.06
Overburden	Upgradient		PZ-3S	49.15	141.97			48.26	142.86	39.40	151.72	44.00	147.12	44.21	146.91	44.59	146.53
Bedrock	Downgradient	AP-1	PZ-4D	52.11	138.99	45.83	145.27	51.89	139.21	43.46	147.64	49.14	141.96	48.35	142.75	49.50	141.60
Overburden	Downgradient	AP-1	PZ-4S	30.75	160.45			27.75	163.45	22.79	168.41	26.95	164.25	27.40	163.80	23.42	167.78
Bedrock	Downgradient		PZ-5D														
Overburden	Downgradient		PZ-6S	26.88	162.59	14.09	175.38	24.47	165.00	16.95	172.52	18.38	171.09	20.36	169.11	18.83	170.64
Bedrock	Downgradient		PZ-7D	35.95	137.13	26.90	146.18	36.21	136.87	29.00	144.08	34.21	138.87	31.28	141.80	34.23	138.85
Overburden	Downgradient		PZ-7S	34.87	138.23	26.82	146.28	34.98	138.12	28.91	144.19	34.15	138.95	31.21	141.89	34.12	138.98
Bedrock	Downgradient		PZ-8D	32.75	137.60	24.74	145.61	33.79	136.56	27.36	142.99	32.83	137.52	28.15	142.20	32.60	137.75
Overburden	Downgradient		PZ-8S	32.59	138.19	26.61	144.17	32.57	138.21	19.39	151.39	27.48	143.30	28.59	142.19	27.63	143.15
Bedrock	Downgradient		PZ-9D	28.74	137.42	20.81	145.35	29.45	136.71	22.74	143.42	28.30	137.86	24.25	141.91	28.02	138.14
Overburden	Downgradient		PZ-9S	28.68	137.34	20.63	145.39	29.12	136.90	22.84	143.18	27.79	138.23	23.92	142.10	27.58	138.44
Bedrock	Downgradient		PZ-10S			29.94	145.69	38.81	136.82	29.29	146.34	35.95	139.68	34.41	141.22	36.42	139.21
Bedrock	Downgradient	AP-1	PZ-11S														
Bedrock	Downgradient	AP-1	PZ-12S	36.22	137.70	27.78	146.14	36.92	137.00	29.84	144.08	35.67	138.25	31.91	142.01	35.27	138.65
Overburden	Downgradient	AP-1	PZ-13S	36.22	137.00	26.24	146.98	35.73	137.49	29.23	143.99	34.18	139.04	30.72	142.50	33.65	139.57
Bedrock	Downgradient		PZ-14	45.87	137.59	38.54	144.92	46.05	137.41	37.97	145.49	43.75	139.71	41.92	141.54	44.07	139.39
Bedrock	Downgradient		PZ-15	33.60	136.77	24.87	145.50	33.73	136.64	26.86	143.51	32.58	137.79	28.15	142.22	32.32	138.05
Bedrock	Downgradient		PZ-16	36.63	137.29	28.62	145.30	37.15	136.77	28.78	145.14	34.52	139.40	32.69	141.23	34.62	139.30
Bedrock	Downgradient		PZ-17	35.56	137.35	26.43	146.48	35.95	136.96	28.11	144.80	33.43	139.48	30.68	142.23	33.65	139.26
Bedrock	Downgradient	AP-1	PZ-18	32.68	137.43	24.22	145.89	33.06	137.05	25.81	144.30	31.16	138.95	27.97	142.14	30.88	139.23
Bedrock	Downgradient	AP-1	PZ-19	34.47	137.58	25.88	146.17	34.94	137.11	27.42	144.63	32.88	139.17	30.03	142.02	32.43	139.62
Bedrock	Downgradient	AP-1	PZ-20	35.79	137.65	27.08	146.36	36.20	137.24	28.81	144.63	34.41	139.03	31.07	142.37	34.14	139.30
Bedrock	Downgradient	AP-1	PZ-21	41.31	138.53	33.09	146.75	41.66	138.18	33.94	145.90	39.42	140.42	36.88	142.96	39.66	140.18
Bedrock	Downgradient	AP-1	PZ-22	48.88	138.81	40.83	146.86	48.89	138.80	40.13	147.56	45.93	141.76	44.49	143.20	46.38	141.31
Bedrock	Downgradient	AP-1	PZ-23														
Bedrock	Downgradient	AP-1	PZ-23A	52.95	138.90	47.02	144.83	52.79	139.06	45.15	146.70	50.21	141.64	49.21	142.64	50.46	141.39
Bedrock	Downgradient	AP-1	PZ-24														
Bedrock	Downgradient	AP-1	PZ-24A	56.59	138.38	50.28	144.69	56.68	138.29	49.37	145.60	54.38	140.59	52.96	142.01	54.59	140.38
Bedrock	Downgradient	AP-1	PZ-25	32.91	138.23	23.87	147.27	33.50	137.64	26.15	144.99	31.93	139.21	27.90	143.24	31.64	139.50
Bedrock	Downgradient		PZ-26														
Bedrock	Downgradient		PZ-27	27.31	137.27	19.22	145.36	27.80	136.78	18.07	146.51	25.54	139.04	23.23	141.35	25.91	138.67
Bedrock	Downgradient	AP-1	PZ-28	28.57	137.39	20.15	145.81	29.06	136.90	21.15	144.81	26.42	139.54	24.28	141.68	26.79	139.17
Bedrock	Downgradient	AP-1	PZ-29	35.41	137.77	26.94	146.24	36.18	137.00	29.36	143.82	35.19	137.99	31.13	142.05	34.71	138.47
Bedrock	Upgradient		PZ-31	41.76	141.20	34.72	148.24	41.75	141.21	32.80	150.16	38.68	144.28	37.32	145.64	39.57	143.39
Bedrock	Upgradient		PZ-32	40.70	140.05	30.34	150.41	40.71	140.04	31.41	149.34	37.14	143.61	36.12	144.63	38.30	142.45
Bedrock	Downgradient		PZ-33	54.68	134.93	44.36	145.25	51.87	137.74	44.13	145.48	49.57	140.04	47.79	141.82	49.82	139.79
Bedrock	Downgradient	AP-1	PZ-57	32.02	137.33	23.42	145.93	32.34	137.01	24.89	144.46	30.33	139.02	27.26	142.09	30.23	139.12

APPENDIX C

SPT Blow Counts Comparison

Soil Type and SPT Summary For Dike Materails (2010 Borings, Plant Mitchell Ash Pond 1)

Project Name: Plant Mitchell Ash Pond-1
Project Number: US0037149.0170
Task: 2.4

Prepared by: Prakash Ghimire
Reviewed by: Andrew Fuggle

S. No.	Location	Boring ID	Type	Simplified delineation	Depth (ft) bgs		Mid depth (ft)	Ground Surface Elevation (ft)	Sample Elevation (ft)	Soil Classification	Blow Counts			Blow Count (SPT-N)	REMARKS
					from	to					n1	n2	n3		
1	Crest	AP1-2	SPT	CLAYEY SAND	2.5	4	3.25	191.1	187.85	SC	5	6	7	13	
2	Crest	AP1-2	SPT	SANDY LEAN CLAY	4.5	6	5.25	191.1	185.85	CL	4	5	3	8	
3	Crest	AP1-2	SPT	SANDY LEAN CLAY	7.5	9	8.25	191.1	182.85	CL	4	4	3	7	
4	Crest	AP1-2	SPT	CLAYEY SAND	14.5	16	15.25	191.1	175.85	SC	5	6	8	14	
5	Crest	AP1-2	SPT	CLAYEY SAND	19.5	21	20.25	191.1	170.85	SC	5	4	4	8	
6	Crest	AP1-2	SPT	CLAYEY SAND	24.5	26	25.25	191.1	165.85	SC	6	6	5	11	
7	Crest	AP1-5	SPT	SANDY LEAN CLAY	2.5	4	3.25	190.7	187.45	CL	3	3	5	8	
8	Crest	AP1-5	SPT	SANDY LEAN CLAY	4.5	6	5.25	190.7	185.45	CL	2	2	3	5	
9	Crest	AP1-5	SPT	SILTY SAND	7.5	9	8.25	190.7	182.45	SM	3	2	4	6	
10	Crest	AP1-5	SPT	LEAN CLAY	14.5	16	15.25	190.7	175.45	CL	5	3	4	7	
11	Crest	AP1-5	SPT	LEAN CLAY	19.5	21	20.25	190.7	170.45	CL	3	3	3	6	
													Minimum=	5	
													Maximim=	14	
													Average=	8.5	

Soil Type and SPT Summary For Dike Materails (2010 Borings, Plant Mitchell Ash Pond 2)

Project Name: Plant Mitchell Ash Pond-1
Project Number: US0037149.0170
Task: 2.4

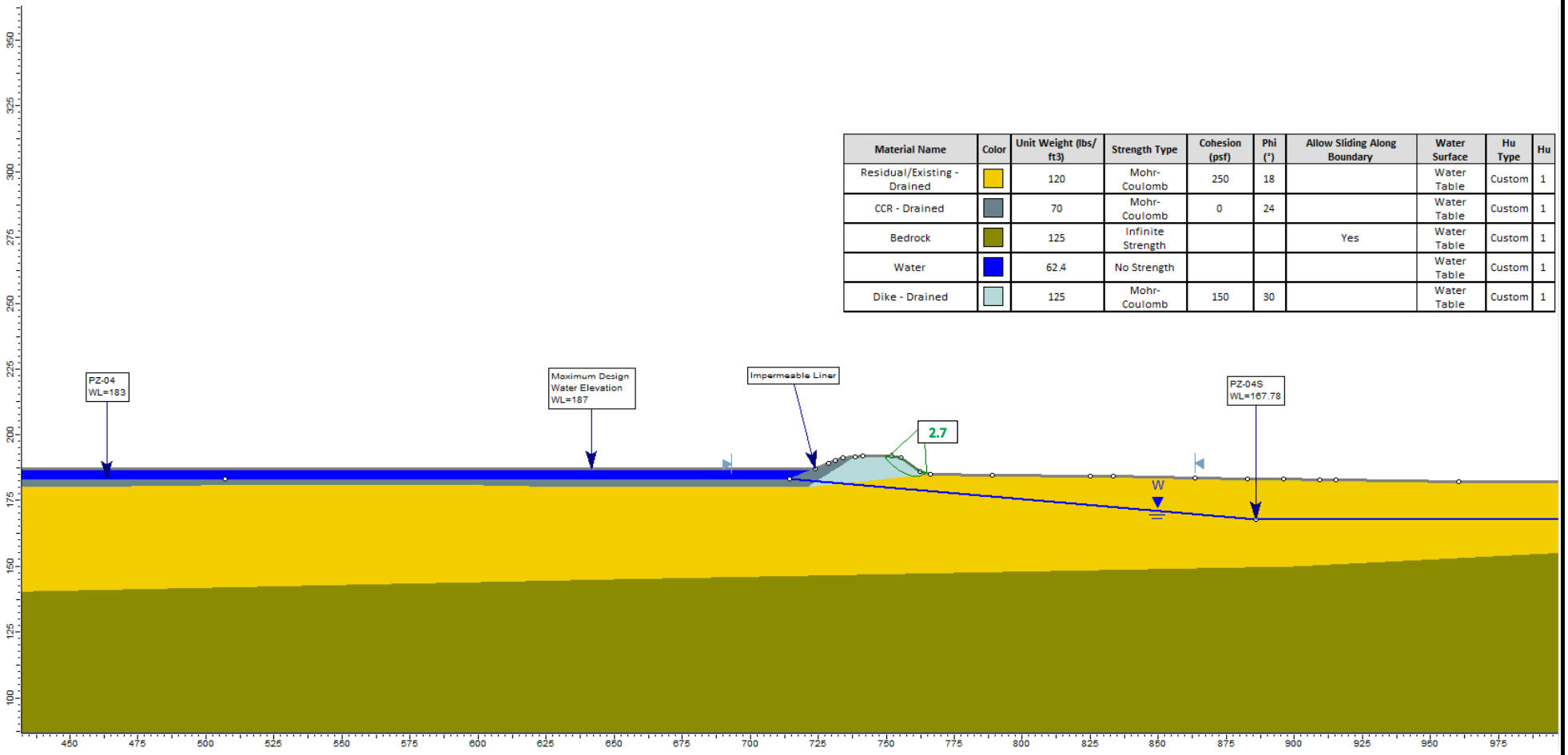
Prepared by: Prakash Ghimire
Reviewed by: Andrew Fuggle

S. No.	Location	Boring ID	Type	Simplified delineation	Depth (ft) bgs		Mid depth (ft)	Ground Surface Elevation (ft)	Sample Elevation (ft)	Soil Classification	SPT			Blow Count (SPT-N)	REMARKS
					from	to					n1	n2	n3		
1	Crest NW	AP2-1	SPT	SANDY FAT CLAY	2.5	4	3.25	194.8	191.55	CH	6	4	8	12	
2	Crest NW	AP2-1	SPT	SANDY FAT CLAY	4.5	6	5.25	194.8	189.55	CH	6	4	8	12	
3	Crest NW	AP2-1	SPT	SANDY FAT CLAY	7.5	9	8.25	194.8	186.55	CH	4	6	7	13	
4	Crest NW	AP2-1	SPT	SILTY SAND	9.5	11	10.25	194.8	184.55	SM	5	9	9	18	
5	Crest NW	AP2-1	SPT	CLAYEY SAND	19.5	21	20.25	194.8	174.55	SC	4	5	9	14	
6	Crest NW	AP2-1	SPT	CLAYEY SAND	24.5	26	25.25	194.8	169.55	SC	5	6	9	15	
7	Crest	AP2-3	SPT	SANDY LEAN CLAY	2.5	4	3.25	193.7	190.45	CL	5	5	7	12	
8	Crest	AP2-3	SPT	SANDY LEAN CLAY	4.5	6	5.25	193.7	188.45	CL	4	4	6	10	
9	Crest	AP2-3	SPT	SANDY LEAN CLAY	7.5	9	8.25	193.7	185.45	CL	5	5	8	13	
10	Crest	AP2-3	SPT	SANDY LEAN CLAY	9.5	11	10.25	193.7	183.45	CL	4	5	7	12	
11	Crest	AP2-3	SPT	SANDY LEAN CLAY	19.5	21	20.25	193.7	173.45	CL	4	3	4	7	
12	Crest	AP2-3	SPT	SANDY LEAN CLAY	24.5	26	25.25	193.7	168.45	CL	3	3	4	7	
13	Crest	AP2-5	SPT	SANDY LEAN CLAY	2.5	4	3.25	194.2	190.95	CL	5	6	9	15	
14	Crest	AP2-5	SPT	SANDY LEAN CLAY	4.5	6	5.25	194.2	188.95	CL	6	7	9	16	
15	Crest	AP2-5	SPT	SANDY LEAN CLAY	7.5	9	8.25	194.2	185.95	CL	7	9	12	21	
16	Crest	AP2-5	SPT	SANDY LEAN CLAY	9.5	11	10.25	194.2	183.95	CL	5	8	12	20	
17	Crest	AP2-5	SPT	CLAYEY SAND	19.5	21	20.25	194.2	173.95	SC	4	7	11	18	
18	Crest	AP2-5	SPT	CLAYEY SAND	24.5	26	25.25	194.2	168.95	SC	5	7	10	17	
													Minimum=	7	
													Maximim=	21	
													Average=	14	

APPENDIX D

Slope Stability Model Outputs

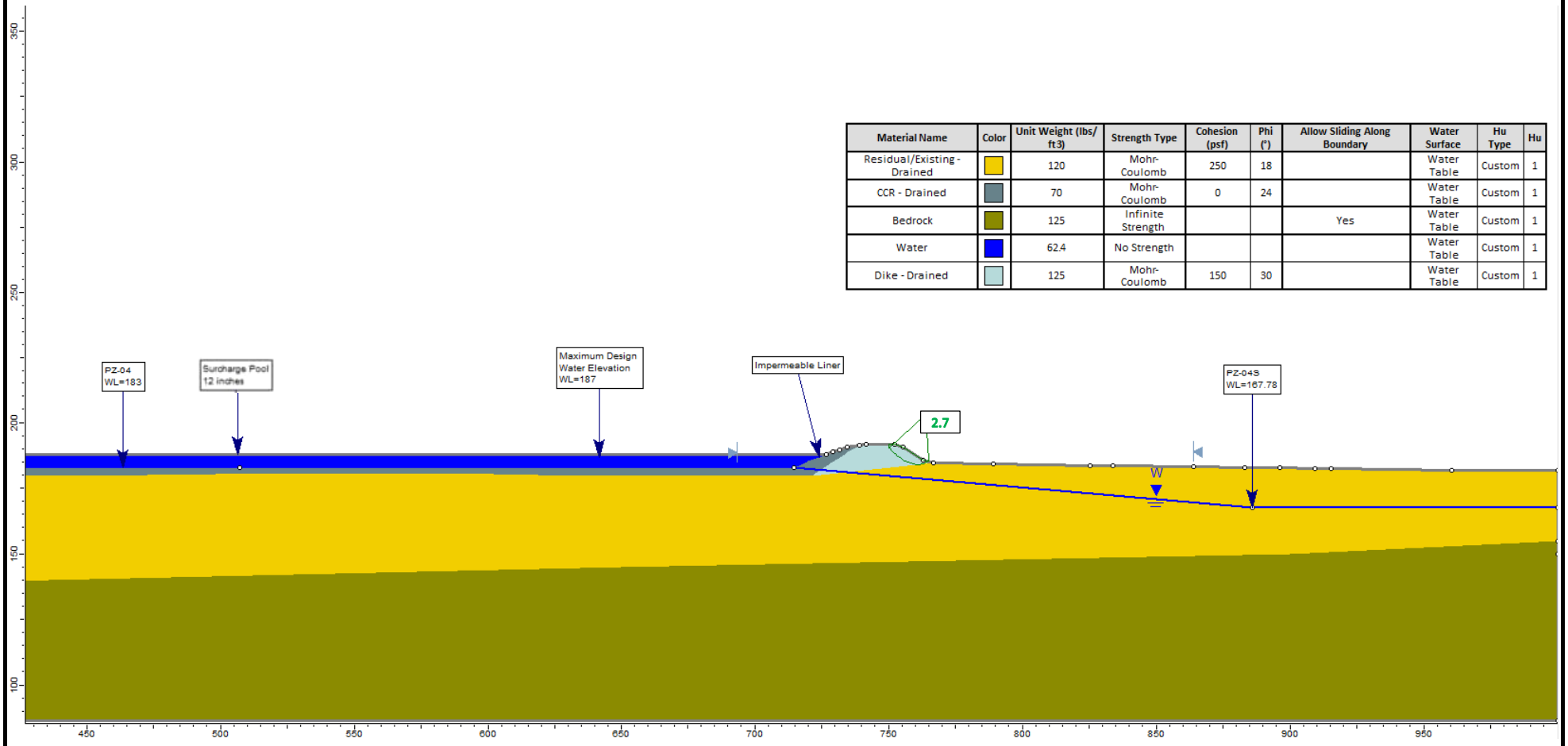
Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Residual/Existing - Drained		120	Mohr-Coulomb	250	18		Water Table	Custom	1
CCR - Drained		70	Mohr-Coulomb	0	24		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Water		62.4	No Strength				Water Table	Custom	1
Dike - Drained		125	Mohr-Coulomb	150	30		Water Table	Custom	1



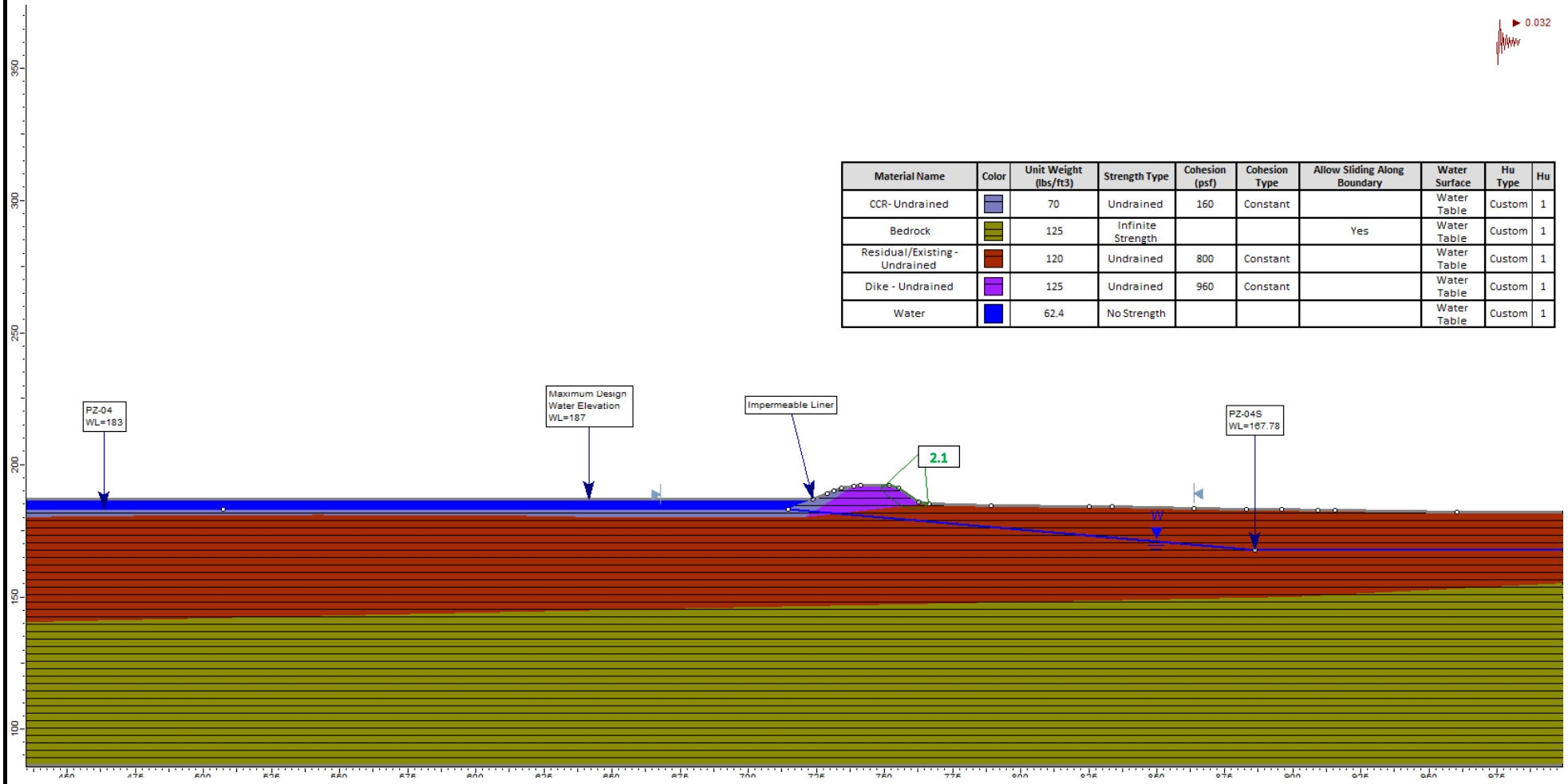
wsp

SCALE	AS SHOWN	PROJECT	**Plant Mitchell Ash Pond-1 (AP-1)**
DATE	Feb 2026	TITLE	**Section A-A'**
MADE BY	PG		
CAD	-	ANALYSIS CONDITION	**Long Term - Drained - Storage Pool**
FILE	INITIAL FACTOR OF SAFETY AP-1	CHECK	AF
PROJECT No.	US0037149.0170	REV. 0	REVIEW RM
CLIENT	**Southern Company Services**	FIGURE	**A-1**

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Residual/Existing - Drained		120	Mohr-Coulomb	250	18		Water Table	Custom	1
CCR - Drained		70	Mohr-Coulomb	0	24		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Water		62.4	No Strength				Water Table	Custom	1
Dike - Drained		125	Mohr-Coulomb	150	30		Water Table	Custom	1



wsp	SCALE	AS SHOWN	PROJECT: Plant Mitchell Ash Pond-1 (AP-1)
	DATE	Feb 2026	TITLE: Section A-A'
	MADE BY	PG	
	CAD	-	ANALYSIS CONDITION: Long Term - Drained - Surcharge Pool
FILE: INITIAL FACTOR OF SAFETY AP-1	CHECK	AF	CLIENT: Southern Company Services
PROJECT No. US0037149.0170 REV. 0	REVIEW	RM	FIGURE: A-2

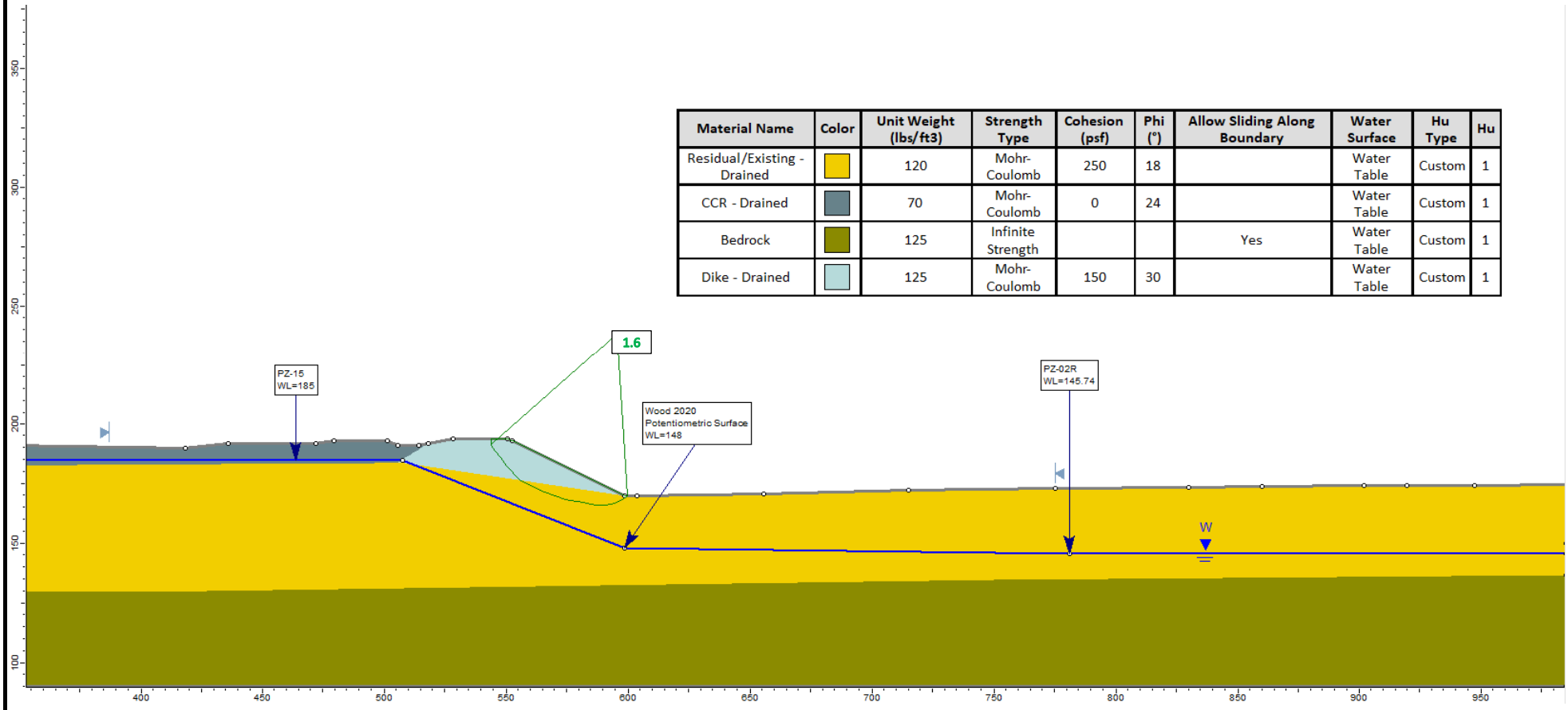


Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Cohesion Type	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
CCR- Undrained		70	Undrained	160	Constant		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Residual/Existing - Undrained		120	Undrained	800	Constant		Water Table	Custom	1
Dike - Undrained		125	Undrained	960	Constant		Water Table	Custom	1
Water		62.4	No Strength				Water Table	Custom	1

Note: Hatching indicates material properties discounted by 20% to account for dynamic excess pore pressures.

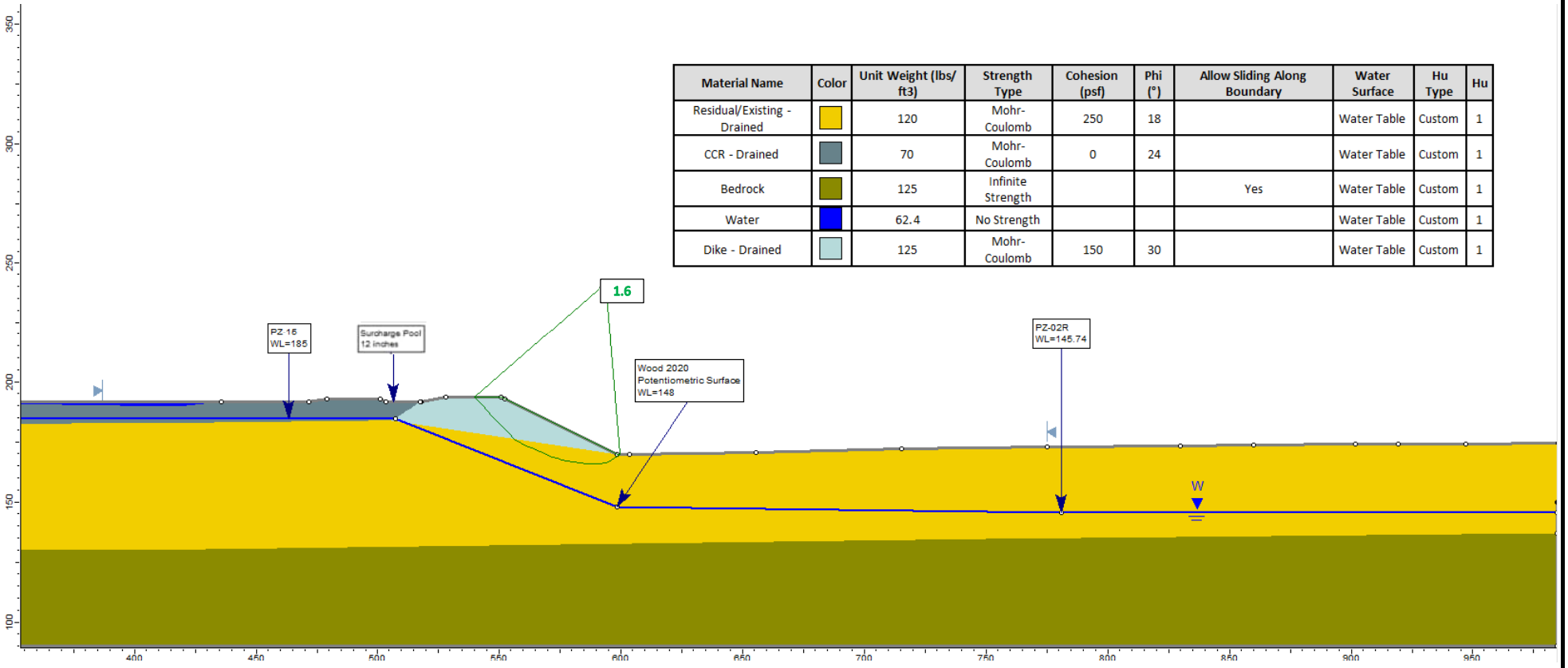
SCALE	AS SHOWN	PROJECT	**Plant Mitchell Ash Pond-1 (AP-1)**
DATE	Feb 2026	TITLE	**Section A-A'**
MADE BY	PG		
CAD	-	ANALYSIS CONDITION	**Seismic Loading**
FILE	INITIAL FACTOR OF SAFETY AP-1	CHECK	AF
PROJECT No.	US0037149.0170	REV. 0	REVIEW RM
CLIENT	**Southern Company Services**	FIGURE	**A-3**

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Residual/Existing - Drained		120	Mohr-Coulomb	250	18		Water Table	Custom	1
CCR - Drained		70	Mohr-Coulomb	0	24		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Dike - Drained		125	Mohr-Coulomb	150	30		Water Table	Custom	1

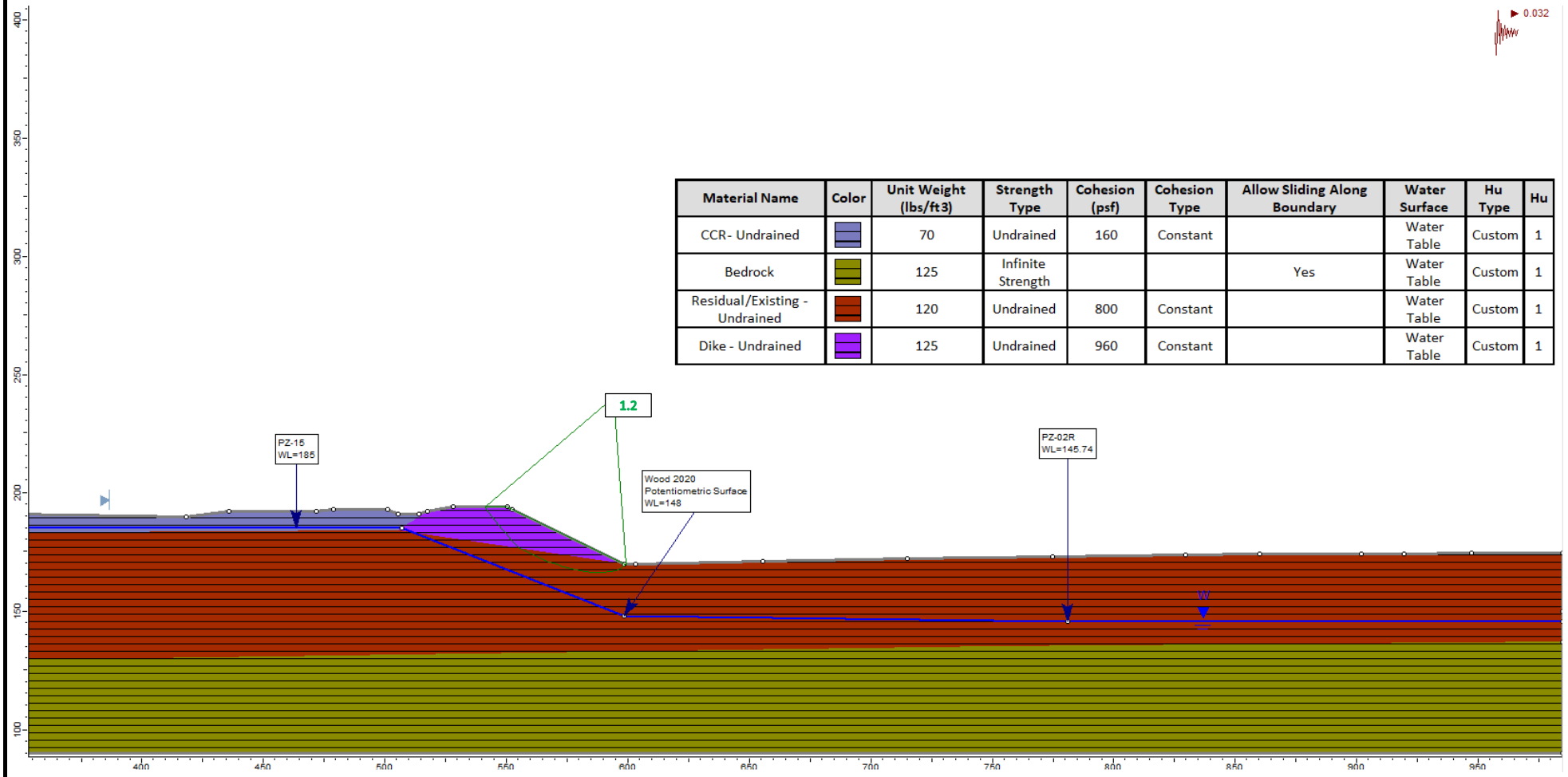


wsp	SCALE	AS SHOWN	PROJECT: Plant Mitchell Ash Pond-1 (AP-1)
	DATE	Feb 2026	TITLE: Section B-B'
	MADE BY	PG	
	CAD	-	ANALYSIS CONDITION: Long Term - Drained - Storage Pool
FILE: INITIAL FACTOR OF SAFETY AP-1	CHECK	AF	CLIENT: Southern Company Services
PROJECT No.: US0037149.0170 REV.: 0	REVIEW	RM	FIGURE: B-1

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Residual/Existing - Drained		120	Mohr-Coulomb	250	18		Water Table	Custom	1
CCR - Drained		70	Mohr-Coulomb	0	24		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Water		62.4	No Strength				Water Table	Custom	1
Dike - Drained		125	Mohr-Coulomb	150	30		Water Table	Custom	1



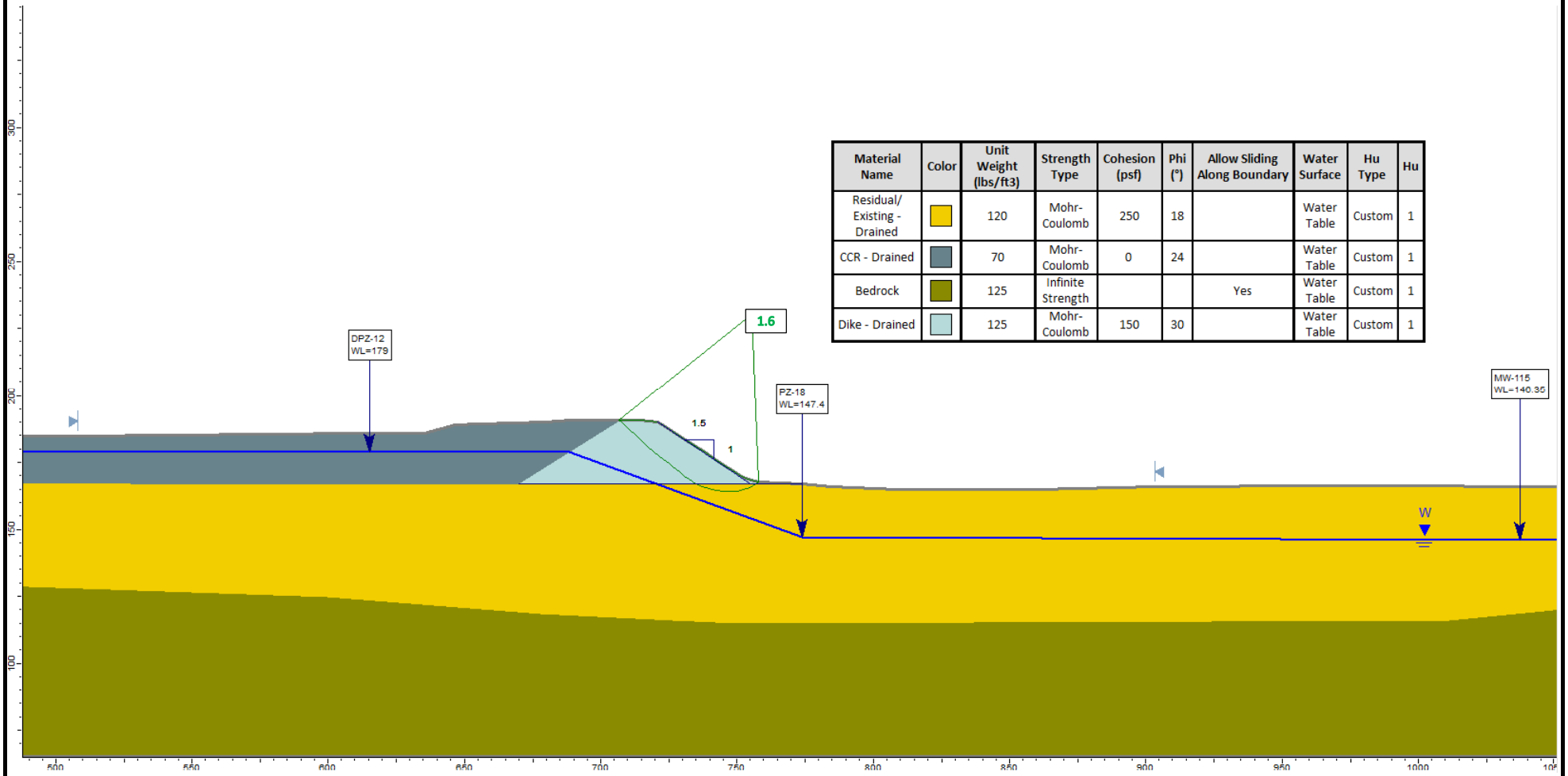
SCALE	AS SHOWN	PROJECT	**Plant Mitchell Ash Pond-1 (AP-1)**
DATE	Feb 2026	TITLE	**Section B-B'**
MADE BY	PG		
CAD	-	ANALYSIS CONDITION	**Long Term - Drained - Surcharge Pool**
FILE	INITIAL FACTOR OF SAFETY AP-1	CHECK	AF
PROJECT No.	US0037149.0170	REV. 0	REVIEW RM
CLIENT	**Southern Company Services**	FIGURE	**B-2**



Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Cohesion Type	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
CCR- Undrained		70	Undrained	160	Constant		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Residual/Existing - Undrained		120	Undrained	800	Constant		Water Table	Custom	1
Dike - Undrained		125	Undrained	960	Constant		Water Table	Custom	1

Note: Hatching indicates material properties discounted by 20% to account for dynamic excess pore pressures.

wsp	SCALE	AS SHOWN	PROJECT: Plant Mitchell Ash Pond-1 (AP-1)
	DATE	Feb 2026	TITLE: Section B-B'
	MADE BY	PG	
	CAD	-	ANALYSIS CONDITION: Seismic Loading
FILE: INITIAL FACTOR OF SAFETY AP-1	CHECK	AF	CLIENT: Southern Company Services
PROJECT No. US0037149.0170 REV. 0	REVIEW	RM	FIGURE: B-3

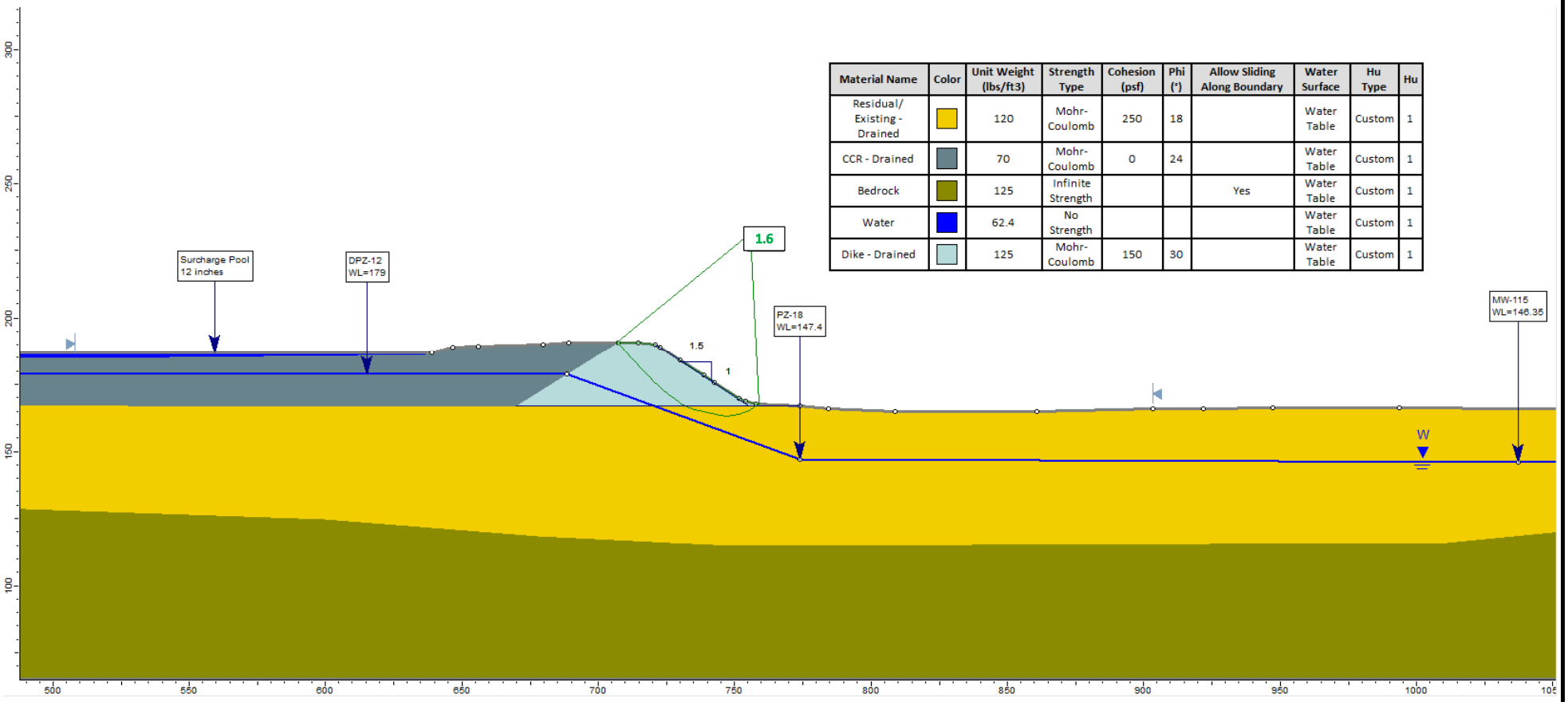


Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Residual/ Existing - Drained		120	Mohr-Coulomb	250	18		Water Table	Custom	1
CCR - Drained		70	Mohr-Coulomb	0	24		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Dike - Drained		125	Mohr-Coulomb	150	30		Water Table	Custom	1

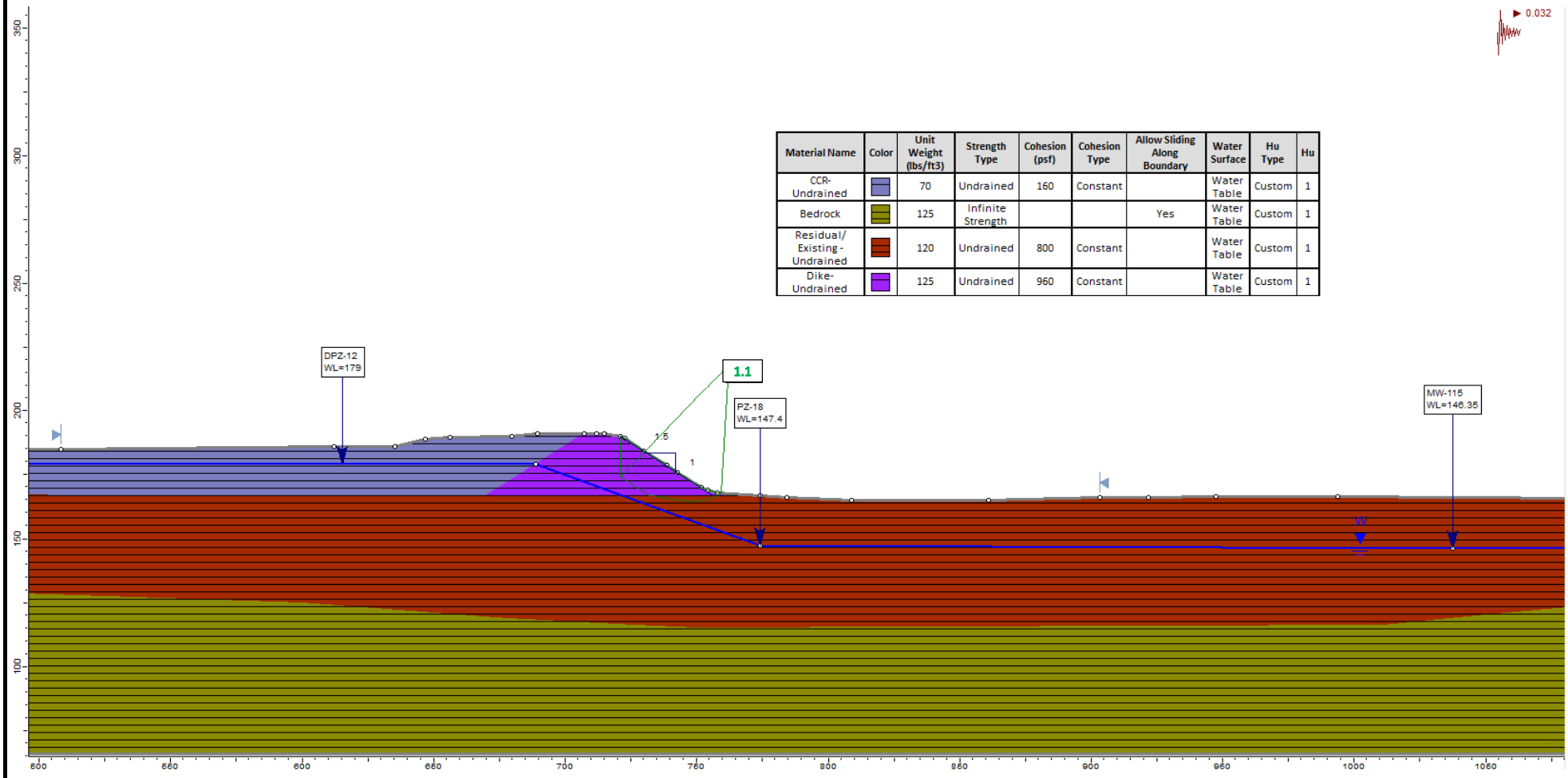
wsp

SCALE	AS SHOWN	PROJECT	Plant Mitchell Ash Pond-1 (AP-1)
DATE	Feb 2026	TITLE	Section C-C'
MADE BY	PG		
CAD	-	ANALYSIS CONDITION	Long Term - Drained - Storage Pool
FILE	INITIAL FACTOR OF SAFETY AP-1	CHECK	AF
PROJECT No.	US0037149.0170	REV. 0	REVIEW RM
CLIENT	Southern Company Services	FIGURE	C-1

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Residual/ Existing - Drained		120	Mohr-Coulomb	250	18		Water Table	Custom	1
CCR - Drained		70	Mohr-Coulomb	0	24		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Water		62.4	No Strength				Water Table	Custom	1
Dike - Drained		125	Mohr-Coulomb	150	30		Water Table	Custom	1



SCALE	AS SHOWN	PROJECT	Plant Mitchell Ash Pond-1 (AP-1)
DATE	Feb 2026	TITLE	Section C-C'
MADE BY	PG		
CAD	-	ANALYSIS CONDITION	Long Term - Drained - Surcharge Pool
FILE	INITIAL FACTOR OF SAFETY AP-1	CHECK	AF
PROJECT No.	US0037149.0170	REV. 0	REVIEW RM
CLIENT	Southern Company Services	FIGURE	C-2

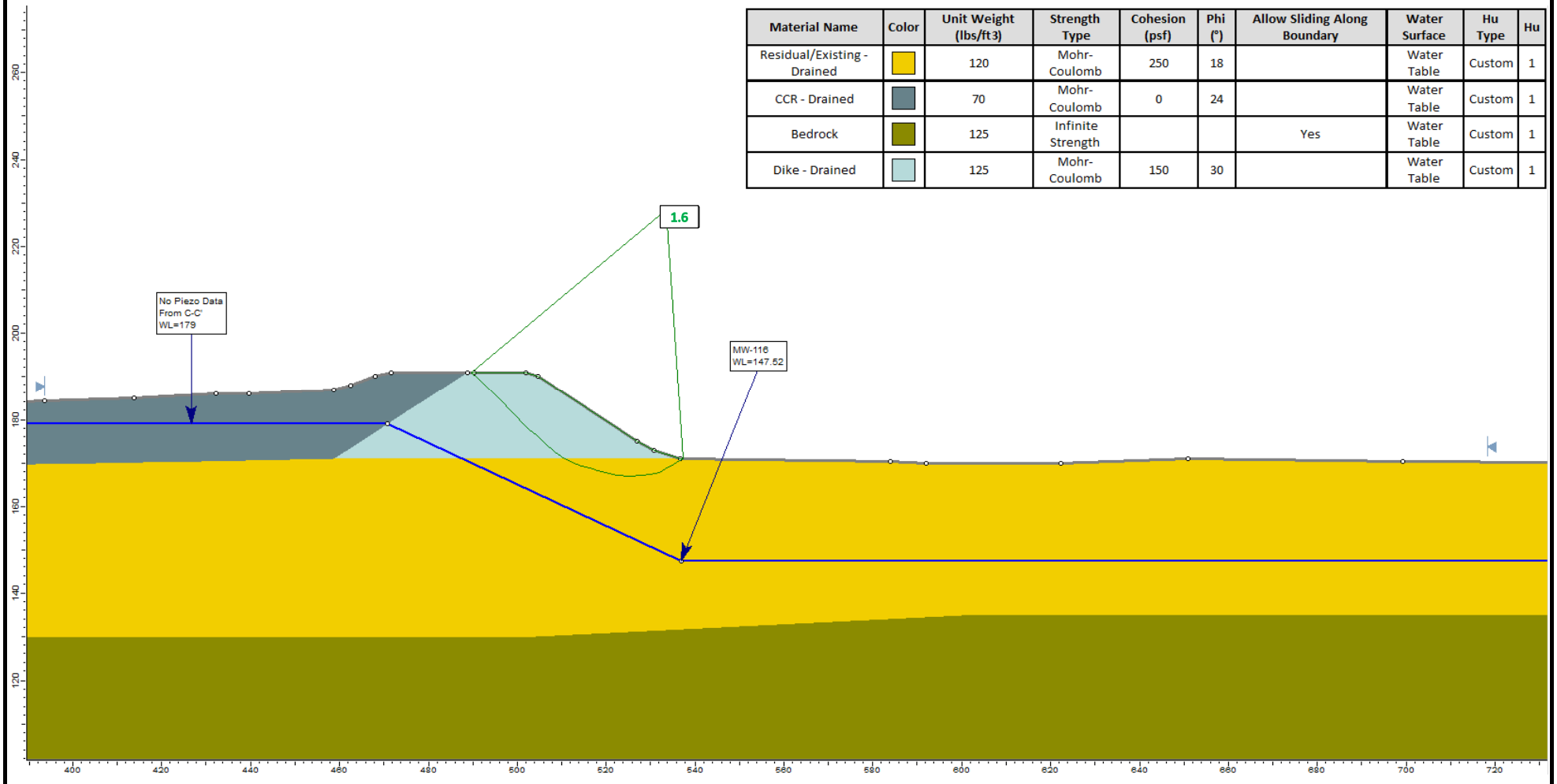


Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Cohesion Type	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
CCR-Undrained		70	Undrained	160	Constant		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Residual/ Existing - Undrained		120	Undrained	800	Constant		Water Table	Custom	1
Dike-Undrained		125	Undrained	960	Constant		Water Table	Custom	1

Note: Hatching indicates material properties discounted by 20% to account for dynamic excess pore pressures.

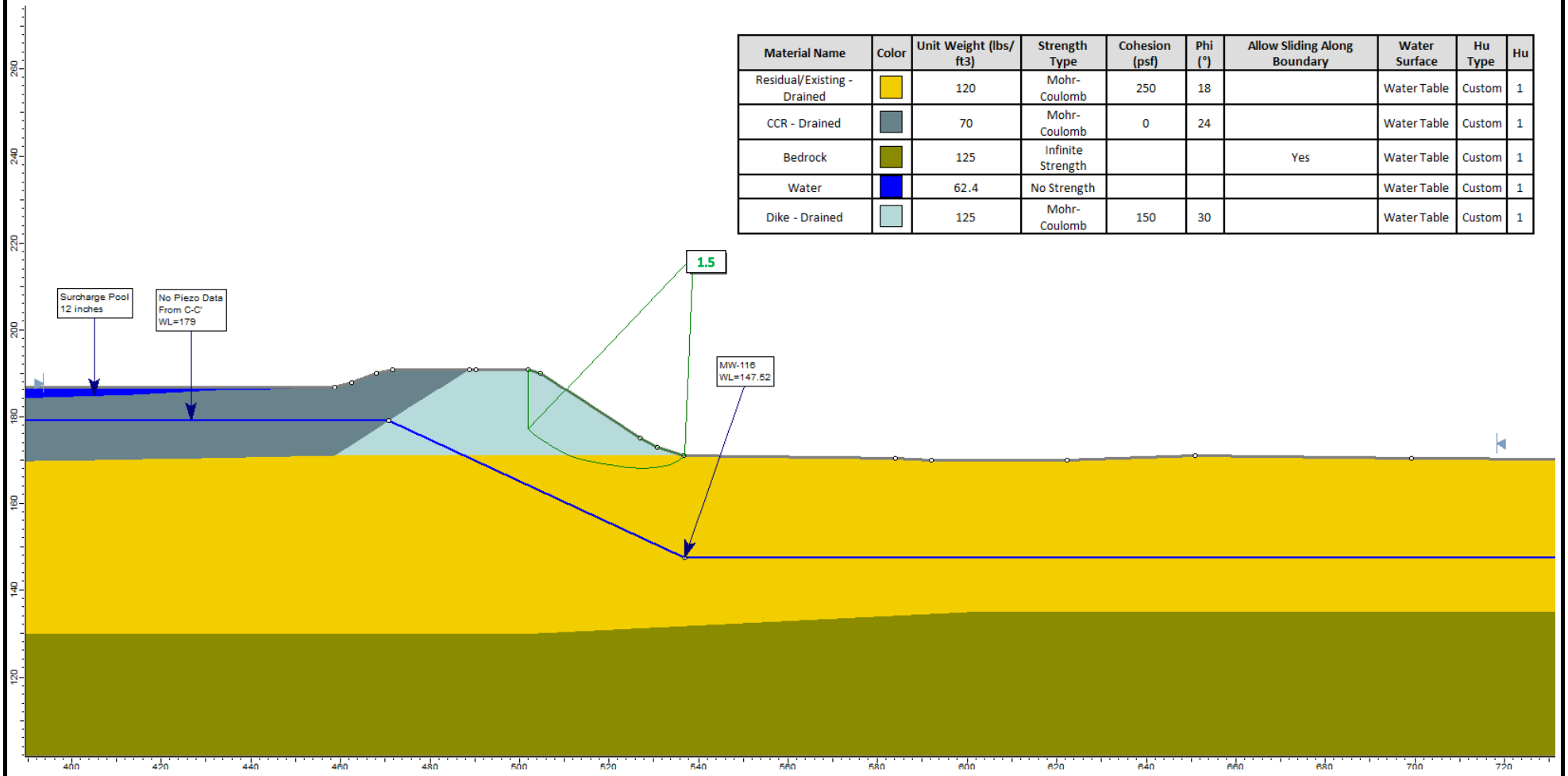
wsp	SCALE	AS SHOWN	PROJECT: Plant Mitchell Ash Pond-1 (AP-1)
	DATE	Feb 2026	TITLE: Section C-C'
	MADE BY	PG	
	CAD	-	ANALYSIS CONDITION: Seismic Loading
FILE: INITIAL FACTOR OF SAFETY AP-1	CHECK	AF	CLIENT: Southern Company Services
PROJECT No.: US0037149.0170 REV.: 0	REVIEW	RM	FIGURE: C-3

Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Residual/Existing - Drained		120	Mohr-Coulomb	250	18		Water Table	Custom	1
CCR - Drained		70	Mohr-Coulomb	0	24		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Dike - Drained		125	Mohr-Coulomb	150	30		Water Table	Custom	1

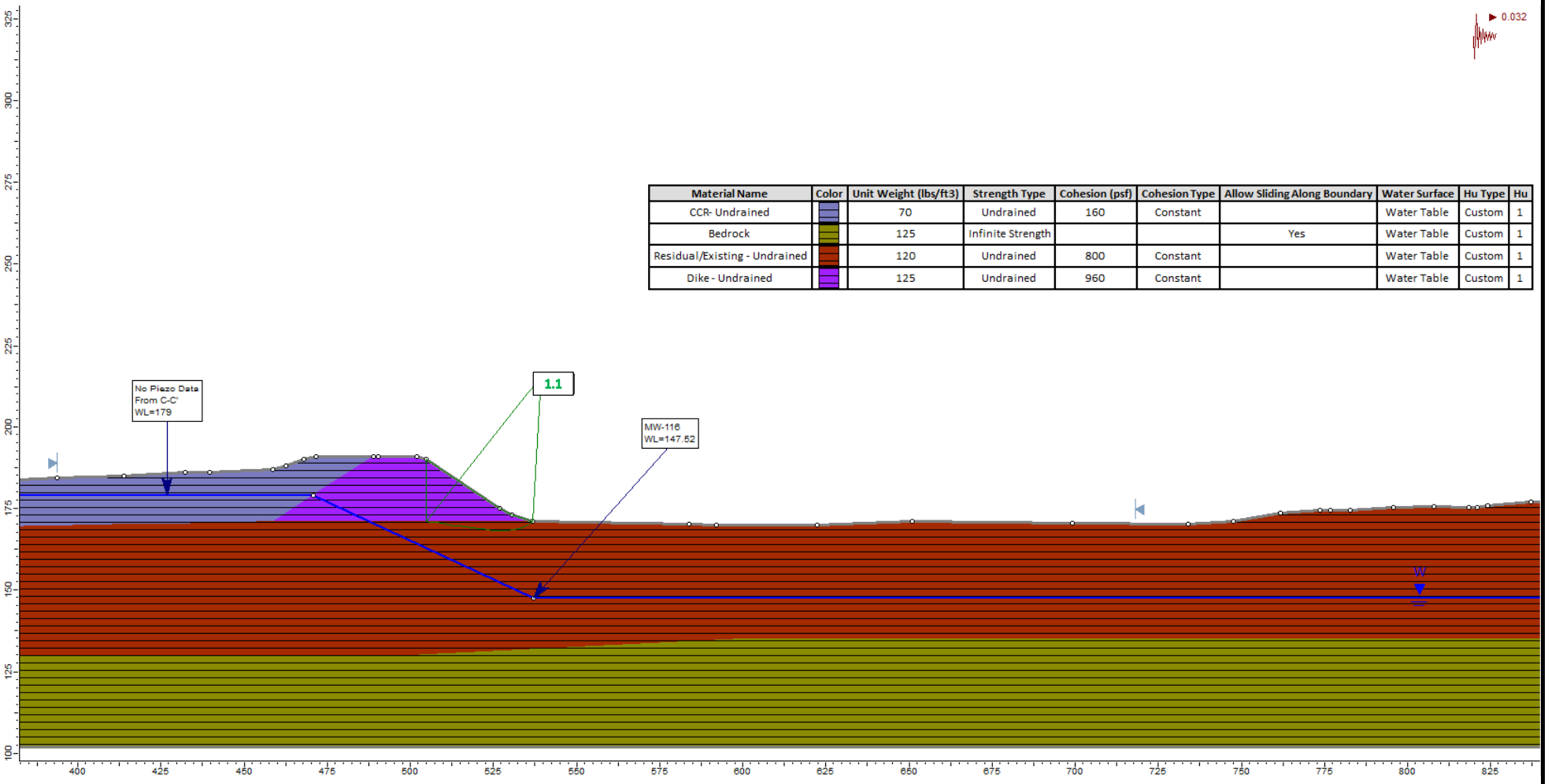


wsp	SCALE	AS SHOWN	PROJECT: Plant Mitchell Ash Pond-1 (AP-1)
	DATE	Feb 2026	TITLE: Section D-D'
	MADE BY	PG	
	CAD	-	ANALYSIS CONDITION: Long Term - Drained - Storage Pool
FILE: INITIAL FACTOR OF SAFETY AP-1	CHECK	AF	CLIENT: Southern Company Services
PROJECT No. US0037149.0170 REV. 0	REVIEW	RM	FIGURE: D-1

Material Name	Color	Unit Weight (lbs/ ft3)	Strength Type	Cohesion (psf)	Phi (°)	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
Residual/Existing - Drained		120	Mohr-Coulomb	250	18		Water Table	Custom	1
CCR - Drained		70	Mohr-Coulomb	0	24		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Water		62.4	No Strength				Water Table	Custom	1
Dike - Drained		125	Mohr-Coulomb	150	30		Water Table	Custom	1



wsp	SCALE	AS SHOWN	PROJECT: Plant Mitchell Ash Pond-1 (AP-1)
	DATE	Feb 2026	TITLE: Section D-D'
	MADE BY	PG	
	CAD	-	ANALYSIS CONDITION: Long Term - Drained - Surcharge Pool
FILE: INITIAL FACTOR OF SAFETY AP-1	CHECK	AF	CLIENT: Southern Company Services
PROJECT No. US0037149.0170 REV. 0	REVIEW	RM	FIGURE: D-2



Material Name	Color	Unit Weight (lbs/ft3)	Strength Type	Cohesion (psf)	Cohesion Type	Allow Sliding Along Boundary	Water Surface	Hu Type	Hu
CCR- Undrained		70	Undrained	160	Constant		Water Table	Custom	1
Bedrock		125	Infinite Strength			Yes	Water Table	Custom	1
Residual/Existing - Undrained		120	Undrained	800	Constant		Water Table	Custom	1
Dike - Undrained		125	Undrained	960	Constant		Water Table	Custom	1

Note: Hatching indicates material properties discounted by 20% to account for dynamic excess pore pressures.

SCALE	AS SHOWN	PROJECT	**Plant Mitchell Ash Pond-1 (AP-1)**
DATE	Feb 2026	TITLE	**Section D-D'**
MADE BY	PG		
CAD	-	ANALYSIS CONDITION	**Seismic Loading**
FILE	INITIAL FACTOR OF SAFETY AP-1	CHECK	AF
PROJECT No.	US0037149.0170	REV. 0	REVIEW RM
CLIENT	**Southern Company Services**	FIGURE	**D-3**

APPENDIX E

Liquefaction Screening

Liquefaction Screening Results

Compositional Screening Criteria (Bray and Sancio, 2006)