2026 INFLOW DESIGN FLOOD CONTROL SYSTEM PLAN
391-3-4-.10(5) and 40 C.F.R. §257.82
PLANT SCHERER ASH POND (AP-1)
GEORGIA POWER COMPANY

The Federal CCR Rule, and, for Existing Surface Impoundments where applicable, the Georgia CCR Rule (391-3-4-.10) require the owner or operator of a CCR surface impoundment to design, construct, operate and maintain an inflow design flood control system capable of adequately managing flow during and following the peak discharge of the specified inflow design flood. The owner or operator must prepare a written inflow design flood control system written plan documenting how the inflow design flood control system has been designed and constructed. *See* 40 C.F.R. § 257.82; Ga. Comp. R. & Regs. r. 391-3-4-.10(5)(b). In addition, the Rules require periodic inflow design flood control system plans within 5 years of development of the previous plan. See 40 C.F.R. § 257.82(c)(4); Ga. Comp. R. & Regs. r. 391-3-4-.10(5)(b). The previously provided Inflow Design Flood Control System Plan was performed and provided by Southern Company Services on October 15, 2021.

The existing CCR surface impoundment known as the Plant Scherer Ash Pond (AP-1), is located at Plant Scherer approximately 8 miles northeast of Forsyth, GA. The facility consists of a 550-acre CCR storage area. The Notice of Intent to Initiate Closure for AP-1 was placed in the Operating Record on October 30, 2020, and closure is not anticipated to have negative impacts on the inflow design flood control plan.

The inflow design flood consists of the rainfall that falls within the limits of the surface impoundment and runoff from approximately 221 acres of adjoining watershed. Stormwater is temporarily stored within the limits of AP-1 and managed and conveyed through the AP-1 closure construction stormwater management systems. There is no discharge through the primary spillway. The primary spillway is a morning glory drop-inlet structure located along the west side of AP-1 and it is constructed of 72-inch diameter reinforced concrete pipe. The auxiliary spillway is an 85-foot wide concrete broad-crested weir that flows into a vegetated trapezoidal channel.

The inflow design flood has been calculated using the Natural Resources Conservation Service (NRCS) method, also known as the Soil Conservation Service (SCS) method, using the probable maximum flood (PMF) storm event required for a high hazard potential surface impoundment. Runoff curve number data was determined using Table 2-2A from the Urban Hydrology for Small Watersheds (TR-55). Appendix A and B from the TR-55 were used to determine the rainfall distribution methodology. Precipitation values were determined from National Oceanic and

Atmospheric Administration (NOAA)'s National Weather Service Hydrometeorological Report No. 51 (HMR-51).

The NRCS provided information on the soil characteristics and hydrologic groups present at the site. It was determined that the hydrological group "B" should be used to best reflect the characteristics of the soils on site. This information was placed into Autodesk Storm and Sanitary Analysis 2019 and used to generate appropriate precipitation curves, storm basin routing information, and resulting rating curves to evaluate surface impoundment capacity.

Resulting calculations indicate that the Plant Scherer AP-1 can safely store and pass the inflow design storm. This plan is supported by appropriate engineering calculations which are attached.

The facility is operated subject to and in accordance with § 257.3-3 of EPA's regulations.

I hereby certify that the inflow design flood control system plan meets the requirements of 40 C.F.R. § 257.82.

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10-01-2026

Inflow Design Control System Plan:
Hydrologic and Hydraulic Calculation Summary

for

Plant Scherer Ash Pond

Prepared by:

AECOM Technical Services Inc.

Originator: Chad Sykes 10/01/2026
Chad Sykes Date

Reviewer: 10/01/2026
Nicholas Fischer Date

Approval: Chet Dicke 10/01/2026
Chet Dicke Date

1.0 Purpose of Calculation

The purpose of this report is to demonstrate the hydraulic capacity of the subject CCR impoundment in order to prepare an inflow design flood control plan as required by the United States Environmental Protection Agency's (EPA) final rule for Disposal of CCR from Electric Utilities (EPA 40 CFR 257) and the Georgia CCR Rule (391-3-4-.10).

2.0 Summary of Conclusions

Ash Pond 1 (AP-1) has been designated a High Hazard Potential surface impoundment; therefore, the appropriate design storm is the full Probable Maximum Precipitation (PMP) event. Southern Company has selected a storm length of 24 hours for all inflow design flood control plans. A hydrologic and hydraulic (H&H) model was developed using HEC-HMS v4.13 for AP-1 to assess the hydraulic capacity of the impoundment with respect to this storm. Due to the progression of ongoing closure activities, AP-1 has available storage capacity from free water removal within the North Pond area. In turn, the peak water surface elevation resulting from the 24-hour PMP is well below the principal spillway crest elevation and will not trigger engagement of the spillway system. A summary of the results of the H&H model is presented below in Table 1. Note that all elevation values throughout the document are per the North American Vertical Datum of 1988 (NAVD 88) unless otherwise stated.

Table 1: PMP Event Results on AP-1

Current Pool Elevation (feet)	Top of AP-1 Dam Elevation (feet)	Principal Spillway Crest Elevation (feet)	Peak Water Surface Elevation (feet)	Freeboard* (feet)	Peak Inflow (cfs)	Peak Outflow (cfs)
450.0	504.1	494.6	475.5	19.1	13,385	0

*Freeboard is measured from the spillway crest to the peak water surface elevation

3.0 Methodology

3.1 HYDROLOGIC ANALYSES

The AP-1 Dam is classified as a High Hazard structure, indicating that failure would result in probable loss of human life. AP-1 is required to safely manage the run-off resulting from the PMP storm event. A summary of the design storm parameters and rainfall distribution methodology used in the H&H model is summarized in Table 2 below.

Table 2: Summary of PMP Parameters

Dam Hazard Classification	High
Design Storm	24-hr PMP
Total Rainfall (in)	43.25
Rainfall Source	HMR-51
Storm Distribution	HMR-52

The AP-1 drainage basin was delineated utilizing topographic data generated by AECOM Technical Services, Inc. (AECOM), dated October 2025, as the existing topography following construction work on site and which was still reflective of existing site conditions at the time of this analysis. This topographic data was supplemented by publicly available USGS topographical data of the area for the portion of the watershed not represented in the topographic data from AECOM.

Runoff Curve Numbers (CNs) for land cover were obtained using values contained in the Natural Resource Conservation Services' (NRCS) Technical Release TR-55 where possible. Because a standard CN for coal combustion residuals (CCR) could not be found, a value of 94 was assigned as a conservative approximation of CCR as a newly graded, bare area with soil of hydrologic soil group D. Additionally, the land cover for woods was increased to 65 from the value of 60 found in the NRCS TR-55 to compensate for the gravel roads and other disturbed areas throughout the predominantly wooded areas in the watershed.

Table 3, below, summarizes the pertinent hydrologic characteristics of AP-1. Additionally, found in Appendix A, Figure A-1 illustrates the existing conditions land cover and time of concentration (T_c) flow path, Table A-1 provides a more detailed summary of the CN calculations, and Table A-2 provides a summary of the T_c calculations.

Table 3: Summary of AP-1 Hydrology

Drainage Basin Area (mi^2)	1.27
Runoff Curve Number	85
Hydrologic Methodology	SCS Method
Time of Concentration (minutes)	43.9
Total Runoff Volume (ac-ft)	2,781.5

3.2 HYDRAULIC ANALYSES

The stage-storage relationship for AP-1 was determined using contour data from as-built site conditions. The principal spillway is a morning glory structure located along the west side of AP-1 which is constructed of 72-inch diameter reinforced concrete pipe and has a crest elevation of 494.6 ft. The auxiliary spillway is an 85-foot-wide concrete broad crested weir that flows into a vegetated trapezoidal channel with a 100-foot bottom width and a weir crest elevation of 498.5 ft. As noted in Section 2, due to recent site work performed as part of the closure of AP-1, the normal pool elevation for AP-1 is approximately 450 ft,

and the water surface elevation is primarily controlled by a pump system instead of the original spillway elements. Because of this, the PMP event is not anticipated to activate the principal or auxiliary spillway. Refer to Table 1 in Section 2 for other model results or see Figure A-2 for the elevation-storage relationship of AP-1, including the elevation and storage values associated with the normal pool elevation, the maximum WSE following the 24-hr PMP, and the principal spillway crest elevation.

APPENDIX A: SUPPORTING INFORMATION

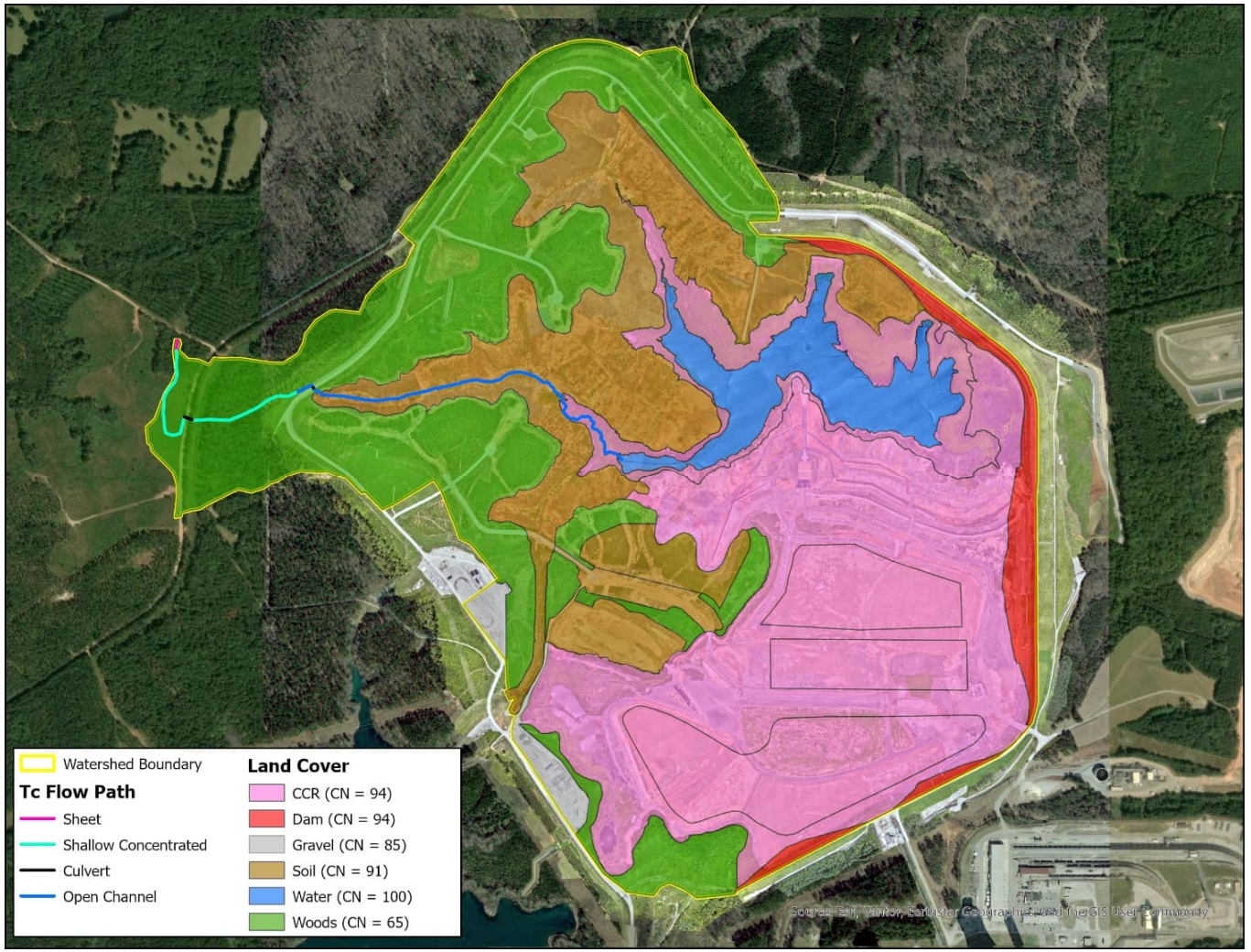


Figure A-1: AP-1 Land Cover and T_c Flow Path

Table A-1: Composite Curve Number Computation

Cover Type	Total Area (ac)	Curve Number
CCR	326.7	94
Soil	168.3	91
Water	41.7	100
Dam	24.1	94
Woods	245.3	65
Gravel	9.9	89

Composite Curve Number:	84.9 (~85)

Table A-2: Time of Concentration and Lag Time Computations

Flow Regime	Flow Length (ft)	Manning's N	Diameter (ft)	Depth (ft)	Top Width (ft)	Bottom Width (ft)	US Elev (ft)	DS Elev (ft)	Slope (ft/ft)	Velocity (ft/s)	Travel Time (hrs)
Sheet	99	0.4	-	-	-	-	563.8	562.6	0.012	0.1	0.40
Shallow Conc.	1,085	Unpaved	-	-	-	-	562.6	531.5	0.029	2.7	0.11
Culvert	76	0.013	1.5	-	-	-	531.5	526.2	0.069	1.6	0.01
Shallow Conc.	1,038	Unpaved	-	-	-	-	526.2	499.5	0.026	2.6	0.11
Open Channel	139	0.033	-	2	22	6	499.5	496.7	0.020	7.4	0.01
Culvert	55	0.013	2	-	-	-	496.7	496.6	0.002	0.3	0.05
Open Channel	3,482	0.03	-	3.5	16	5	496.6	446.2	0.015	9.6	0.10
Total	5,974										0.79

T_c (min):	43.9
T_{lag} (min):	26.4

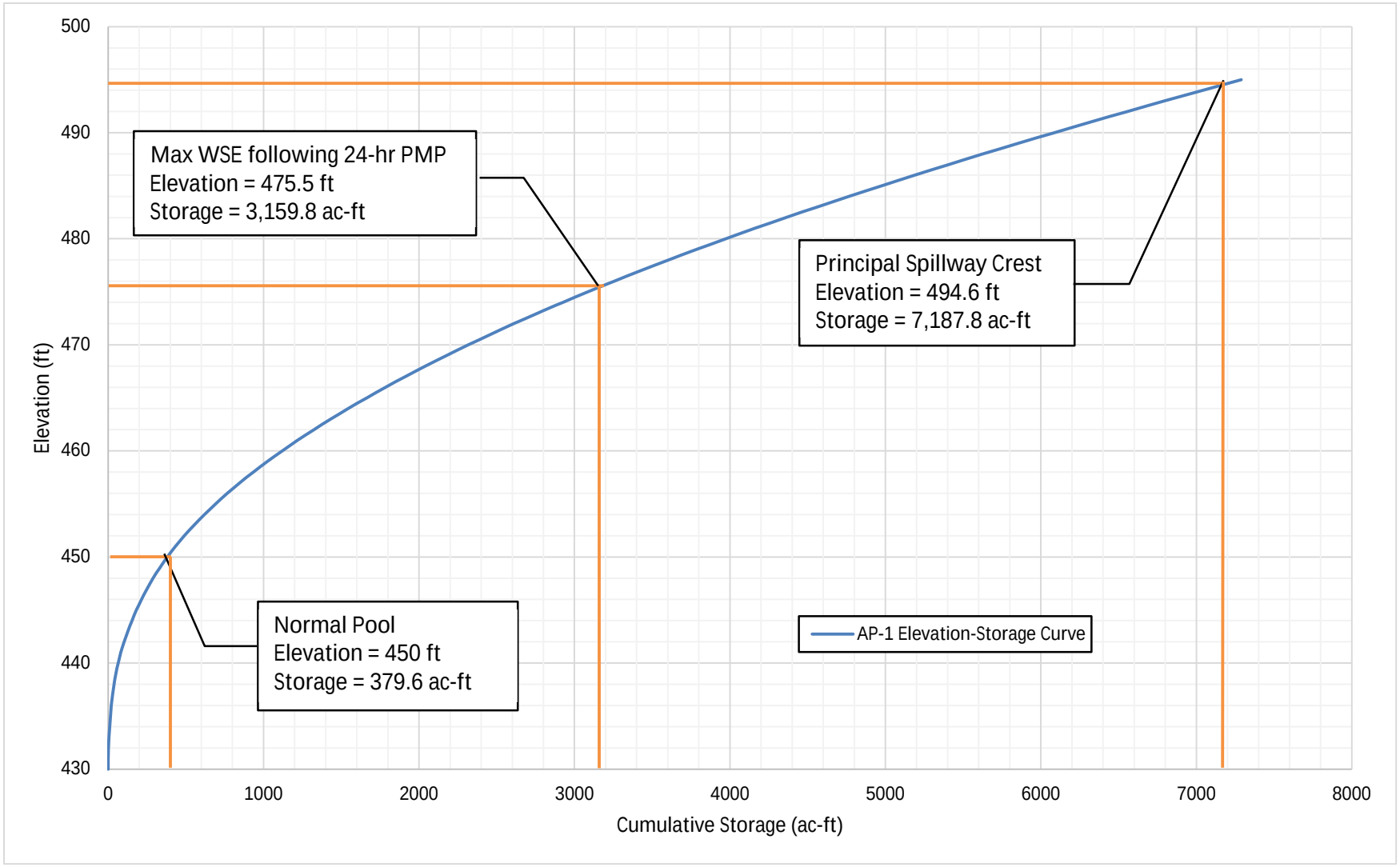


Figure A-2: AP-1 Stage-Storage Curve