PERIODIC STRUCTURAL STABILITY ASSESSMENT
391-3-4-.10(4) and 40 C.F.R. § 257.73
PLANT SCHERER ASH POND (AP-1)
GEORGIA POWER COMPANY

The Federal CCR Rule, and, for Existing Surface Impoundments where applicable, the Georgia CCR Rule (391-3-4-.10) require the owner or operator of an existing CCR surface impoundment to conduct initial and periodic structural stability assessments. The owner or operator must conduct an assessment of the CCR unit and document whether the design, construction, operation, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices for the maximum volume of CCR and CCR wastewater which can be impounded therein. *See* 40 C.F.R. § 257.73(d); Ga. Comp. R. & Regs. r. 391-3-4-.10(4)(b). In addition, the Rules require a subsequent assessment be performed within 5 years of the completion of the previous assessment. *See* 40 C.F.R. § 257.73(f)(3); Ga. Comp. R. & Regs. r. 391-3-4-.10(4)(b). The previous Periodic Structural Stability Assessment was placed in the Operating Record on October 15, 2021.

The CCR surface impoundment known as Plant Scherer Ash Pond (AP-1) is located on Georgia Power's Plant Scherer property, approximately 8 miles northeast of Forsyth, Georgia. The Notification of Intent to Initiate Closure was placed in the Operating Record on October 30, 2020, and closure activities have been initiated. The closure of AP-1 is not anticipated to have any negative impacts on the stability of the perimeter dams.

AP-1 is formed by engineered cross-valley dams. The foundations and abutments generally consist of Piedmont Physiographic Province residual soils consisting of stiff to hard sandy silts and loose to very dense silty sands with varying clay and mica content. A transitional layer of partially weathered rock is present between the residual soils and the underlying bedrock. The bedrock consists primarily of feldspathic biotite gneiss with localized lenses of feldspathic hornblende gneiss/amphibolite.

Slope protection against surface erosion consists of gravel surfaced roads along the crest and benches and well vegetated downstream slopes. Dewatering and CCR removal in the North Slope area along Berry Creek and along the Eastern Drainage Ditch of AP-1 have been completed. The Fabriform along the North Slope has been removed, and the area has been vegetated. Fabriform is still in place along remaining portions of the East Slope Area. Wave action is not a concern at AP-1 due to the revegetation and the Fabriform concrete erosion mat that remains in place. As such, AP-1 is not subjected to rapid drawdown conditions.

The cross-valley dams have been properly constructed using mechanical stabilization and earthen soils compacted to a density sufficient to withstand the range of loading conditions. Inspections of the AP-1 dam are conducted on a regular basis. No actual or potential structural weaknesses of the dam or abutments, nor any existing conditions that would negatively impact the stability of the AP-1 dam and abutments, were documented in the recent inspection reports.

Vegetated slopes of the dams are properly maintained to a height that allows for routine visual inspections. Historical operational performance and inspection reports reveal no indication of slope instability associated with the dams of AP-1.

The principal spillway is a morning glory drop-inlet structure located along the west side of AP-1, and it is constructed of 72-inch diameter reinforced concrete pipe. The auxiliary spillway is an 85-foot wide concrete, broad crested weir that flows into a vegetated trapezoidal channel. The spillways are designed, constructed, operated, and maintained to adequately manage flow during the peak discharge from the probable maximum flood. During closure, stormwater and contact water within AP-1 is segregated and managed by a series of conveyance ditches, temporary sediment ponds, and pumps. Contact water is contained within AP-1 and pumped to the on-site Wastewater Treatment System prior to discharge. Stormwater (non-contact water) is conveyed by gravity flow and pumps to the existing surface water management system. The principal and auxiliary spillways will be removed during closure.

The CCR unit does not have a discharge structure that passes through or under the dam.

A review of current conditions indicates the downstream slopes of the dams are not subject to inundation from adjacent water bodies.

I hereby certify that the structural stability assessment was conducted in accordance with 40 C.F.R. § 257.73(d).



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