PERIODIC STRUCTURAL STABILITY ASSESSMENT
391-3-4-.10(4) and 40 C.F.R. § 257.73
PLANT WANSLEY ASH POND 1 (AP-1)
GEORGIA POWER COMPANY

The Federal CCR Rule, and, for Existing Surface Impoundments where applicable, the Georgia CCR Rule (391-3-4-.10) require the owner or operator of an existing CCR surface impoundment to conduct initial and periodic structural stability assessments. The owner or operator must conduct an assessment of the CCR unit and document whether the design, construction, operation, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices for the maximum volume of CCR and CCR wastewater which can be impounded therein. *See* 40 C.F.R. § 257.73(d); Ga. Comp. R. & Regs. r. 391.3-4-.10(4)(b). In addition, the Rules require a subsequent assessment be performed within 5 years of the completion of the previous assessment. *See* 40 C.F.R. § 257.73(f)(3); Ga. Comp. R. & Regs. r. 391.3-4-.10(4)(b). The previous Periodic Structural Stability Assessment was placed in the Operating Record on October 15, 2021.

The CCR surface impoundment known as Plant Wansley AP-1 is located on Plant Wansley property in Heard County, Georgia near Carrollton. The Notification of Intent to Initiate Closure was placed in the Operating Record on April 17, 2019, and revised February 5, 2024, to indicate that the closure strategy for AP-1 had changed from closure in place to closure by removal of CCR. Closure activities are currently underway. The closure has been designed to have no negative impacts on the stability of the perimeter embankments.

AP-1 is formed by an engineered cross-valley dam on the northeast side of the impoundment. This dam is also referred to as the "separator dike" as it separates the ash pond from the Plant Wansley storage water pond. There is also a small dam located at the west end of the impoundment near the impoundment's auxiliary spillway.

The foundations and abutments consist of Piedmont Physiographic Province residual soils consisting of stiff to very stiff silts and clays. A transitional layer of partially weathered rock is typically present between the residual soils and the underlying bedrock. The bedrock consists primarily of graphitic schist, biotite schist, schist with interlayered mafic units, amphibolite/hornblende gneiss, granitic gneiss, and feldspathic quartzite.

Slope protection against surface erosion consists of crushed stone. Wave action is not a concern at this site due to the characteristics of the impoundment and the riprap sized crushed stone used along the upstream and downstream slopes of the dam.

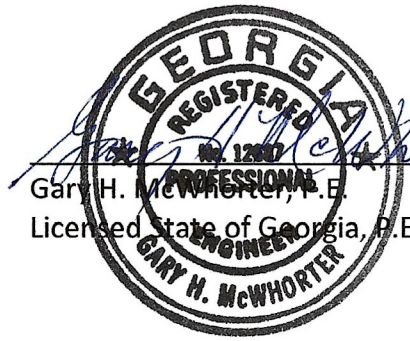
A review of current conditions indicates the downstream slope of the separator dike is subject to inundation from the adjacent storage water pond. The storage water pond operates within a limited normal pool range. The downstream slope of the separator dike was constructed with a protective layer of riprap with a minimum thickness of two feet, and previous stability analyses have indicated that the slope is stable under steady-state conditions and under rapid drawdown conditions from the storage water pond. Historical operational performance and inspection reports reveal no indication of structural instability associated with the separator dike.

The separator dike has been properly constructed using conventional earthwork means and methods and compacted to a density sufficient to withstand the range of loading conditions.

The dewatering process is currently underway in AP-1. Stormwater and contact water that is temporarily stored within the limits of the surface impoundment is routed from the AP-1 discharge structure located at the southwest end of AP-1 to the on-site water treatment facility prior to treatment and discharged through the current outfall structure. The 42-in diameter fiberglass principal spillway located at the southwest end of AP-1 has been sealed to prevent contact water discharge prior to passing through the water treatment facility. An auxiliary spillway system consisting of a 36-in diameter corrugated metal pipe (CMP) and a 45-ft wide

concrete broad crested weir is located on the west end of the impoundment. The auxiliary discharge structure (36-inch CMP) passes through the western embankment. The results of the 2026 Periodic Inflow Design Flood Control System Plan indicate the existing inflow management system provides adequate capacity to safely manage the peak discharge of the inflow design flood without overtopping the perimeter dams. There is no evidence of deterioration, deformation, distortion, bedding deficiencies, sedimentation, and debris which may negatively affect the operation of the structure.

I hereby certify that the structural stability assessment was conducted in accordance with 40 C.F.R. § 257.73 (d).



10/12/26

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